

TAJEL NO. IV.5.
 METODE RATIO JUMLAH KENDARAAN BERMOTOR DENGAN JUMLAH PENDUKUK
 KOTA/KABUPATEN DI SEMARANG, DARI TAHUN 1981 s/d TAHUN 2000

No. Tahun	JUMLAH PENDUKUK	Rasio Jumlah Kendaraan Bermotor dengan Jumlah Penduduk																	
		Angeutan Pemungut Motor Sola 4 (Empat)					U M U N												
		Jumlah	Ratio	Jumlah	Ratio	Jumlah	Ratio	Jumlah	Ratio	Jumlah	Ratio								
1. 1981	1 020 468	141	7880	131	15 037	58	1986	514	312	3271	99	61 714	17	33 098	78	38 407	27	132 121	10
2. 1982	1 050 637	132	8365	125	15 395	54	2064	509	345	3045	95	65 189	16	12 323	73	41 738	25	127 634	10
3. 1983	1 072 324	134	8447	127	16 466	55	2105	509	360	2979	92	78 373	14	15 436	69	43 536	25	151 895	10
4. 1984	1 086 198	135	8471	128	16 513	56	2117	513	375	2897	90	89 771	12	16 415	66	45 052	24	124 803	10
5. 1985	1 103 038	133	8697	128	17 034	65	2161	513	397	2795	87	100 216	11	17 610	63	47 416	23	127 832	8
6. 1986	1 132 297	131	8929	127	17 572	64	2206	513	420	2696	84	111 901	10	18 838	60	49 921	23	154 822	7
7. 1987	1 156 075	129	9167	128	18 127	64	2252	513	444	2603	81	124 943	9	20 260	57	52 577	22	177 526	7
8. 1988	1 180 352	127	9412	125	18 701	63	2298	513	470	2511	79	139 518	8	21 731	54	55 334	21	174 912	6
9. 1989	1 205 133	125	8663	125	19 293	62	2347	513	497	2425	76	155 736	8	23 309	52	58 380	21	214 166	6
10. 1990	1 230 446	123	8921	124	19 904	62	2396	514	526	2339	74	173 930	7	25 001	49	61 347	20	235 437	5
11. 1991	1 255 295	121	10185	123	20 535	61	2446	514	556	2260	72	194 233	6	26 816	47	64 907	19	259 140	5
12. 1992	1 282 666	120	10458	123	21 187	61	2497	514	588	2181	69	215 331	6	29 783	45	68 474	19	235 355	4
13. 1993	1 309 602	119	10737	122	21 840	60	2549	514	622	2105	67	222 169	5	30 551	42	72 260	18	214 223	4
14. 1994	1 337 103	116	11024	121	22 555	59	2603	514	658	2032	65	230 106	5	33 091	40	76 281	18	245 687	4
15. 1995	1 365 182	114	11318	121	23 272	59	2658	514	696	1961	63	240 935	5	35 433	38	80 549	17	252 484	4
16. 1996	1 393 870	112	11620	122	24 015	58	2714	514	736	1894	61	257 121	4	38 070	37	85 083	15	272 224	3
17. 1997	1 423 170	110	11930	122	24 776	57	2771	514	779	1827	59	276 472	4	40 833	35	84 893	16	285 321	3
18. 1998	1 453 095	109	12249	122	25 566	57	2829	514	824	1763	57	290 345	3	43 738	33	95 018	15	315 351	3
19. 1999	1 483 918	107	12576	122	26 382	56	2888	514	872	1701	54	299 339	3	46 978	32	100 539	15	329 517	3
20. 2000	1 514 671	106	12912	121	27 235	56	2949	514	923	1641	54	324 203	3	50 359	30	106 244	14	343 653	2
Jumlah	2,10X	3,67%	2,67%	3,17%	2,1%	5,8%	6,08%	4,65%	11,66%	7,26%	10,53%	10,20%							

Subber data : Tabel NO. II.8 dan NO. II.1

LAPORAN HASIL PENYELIDIKAN TANAH
DI P.T. SILIWANGI PLAZA
SEMARANG

PENDAHULUAN

Penyelidikan tanah dilaksanakan terdiri atas 2 titik Boring dan 9 titik Sondir.

Dengan menggunakan alat Boor tangan jenis Auger, kedalaman yang dapat dicapai -7.00 meter.

Dengan menggunakan alat Sondir jenis Dutch Cone Penetrometer berkapasitas 2,5 ton, kedalaman yang dapat dicapai berkisar antara -16.00 meter sampai -22,5 meter.

Samples hasil Boring ditest di Laboratorium untuk mengetahui physical properties & Engineering properties tanah yang bersangkutan. Selanjutnya harap periksa lampiran.

HASIL BORING.

Bagian atas tanahnya berupa jenis clay lunak. Makin kebawah clay mengandung pasir/silt, sedang keadaannya tetap lunak, tidak compact. Kadang-kadang tanahnya mengandung kulit kerang. Keadaan tanah yang lunak ini dapat dilihat pada hasil direct shear test, yaitu harga C dan ϕ - nya sangat kecil.

HASIL SONDIR.

Secara umum dapat dikatakan bahwa Conus resistance sangat kecil, yaitu berkisar $\pm 5 \text{ kg/cm}^2$.

Sampai kedalaman -13.00 meter Conus resistencenya baru agak besar, kemungkinan disini tanahnya sudah compact atau berupa pasir.

Nilai friction pada bagian atas juga kecil sekali. Hal tersebut disebabkan karena tanahnya tidak compact.

SARAN-SARAN/KESIMPULAN

A. Besarnya safe bearing capacity sebagai berikut :

- Pada kedalaman -1.00 m = 0,1 kg/cm²
- Pada kedalaman -2.00 m = 0,1 kg/cm²
- Pada kedalaman -3.00 m = 0,1 kg/cm²
- Pada kedalaman -4.00 m = 0,2 kg/cm²
- Pada kedalaman -5.00 m = 0,2 kg/cm²
- Pada kedalaman -6.00 m = 0,3 kg/cm²
- Pada kedalaman -7.00 m = 0,3 kg/cm²

B. Berhubung

B. Berhubung dengan keadaan tanah yang jelek, maka kami sarankan sebagai berikut ;

Diadakan penggalian sedalam - 3.00 meter, kemudian diisi sirtu (campuran pasir kasar + kerikil/kricak), setebal 1.00 meter .

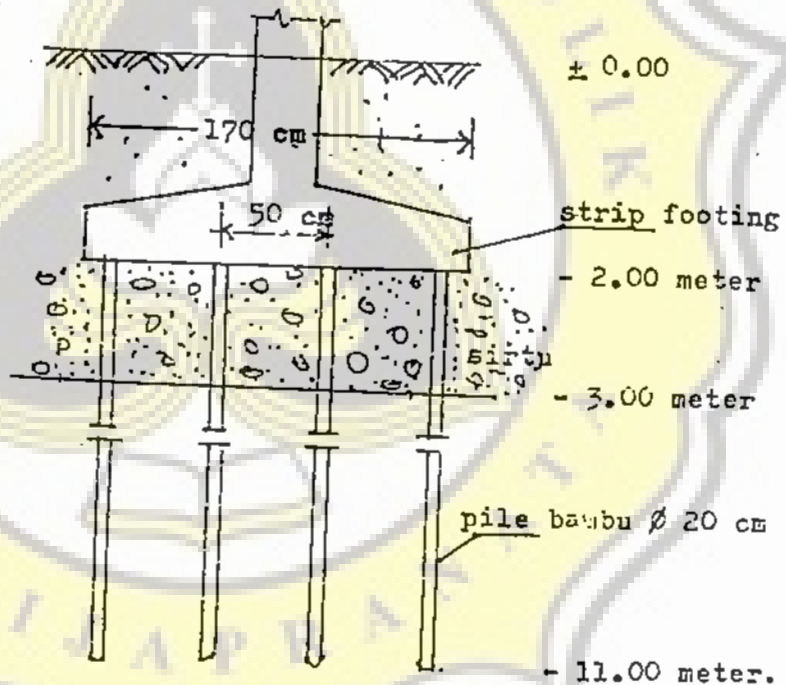
Kemudian dipasang pile dari bambu berdiameter 20 cm, ditanam masuk sampai kedalaman - 11.00 meter dari permukaan tanah asli atau - 9.00 meter dari permukaan sirtu .

Jarak antara masing-masing pile bambu - 0.50 meter .

Diatas pile bambu baru dipasang strip footing .

Dengan lebar strip footing = 1.70 meter, maka untuk setiap meter panjang strip footing dapat menahan beban sebesar = 20 ton. Harga ini belum dikurangi berat sendiri footing .

Pada strip footing ini hendaknya diperhitungkan perataan beban oleh Sloof .



C. Pada harga diatas sudah dimasukkan faktor of safety = 3.00 .

Semarang, 6 Oktober 1983

Laboratorium Mekanika Tanah

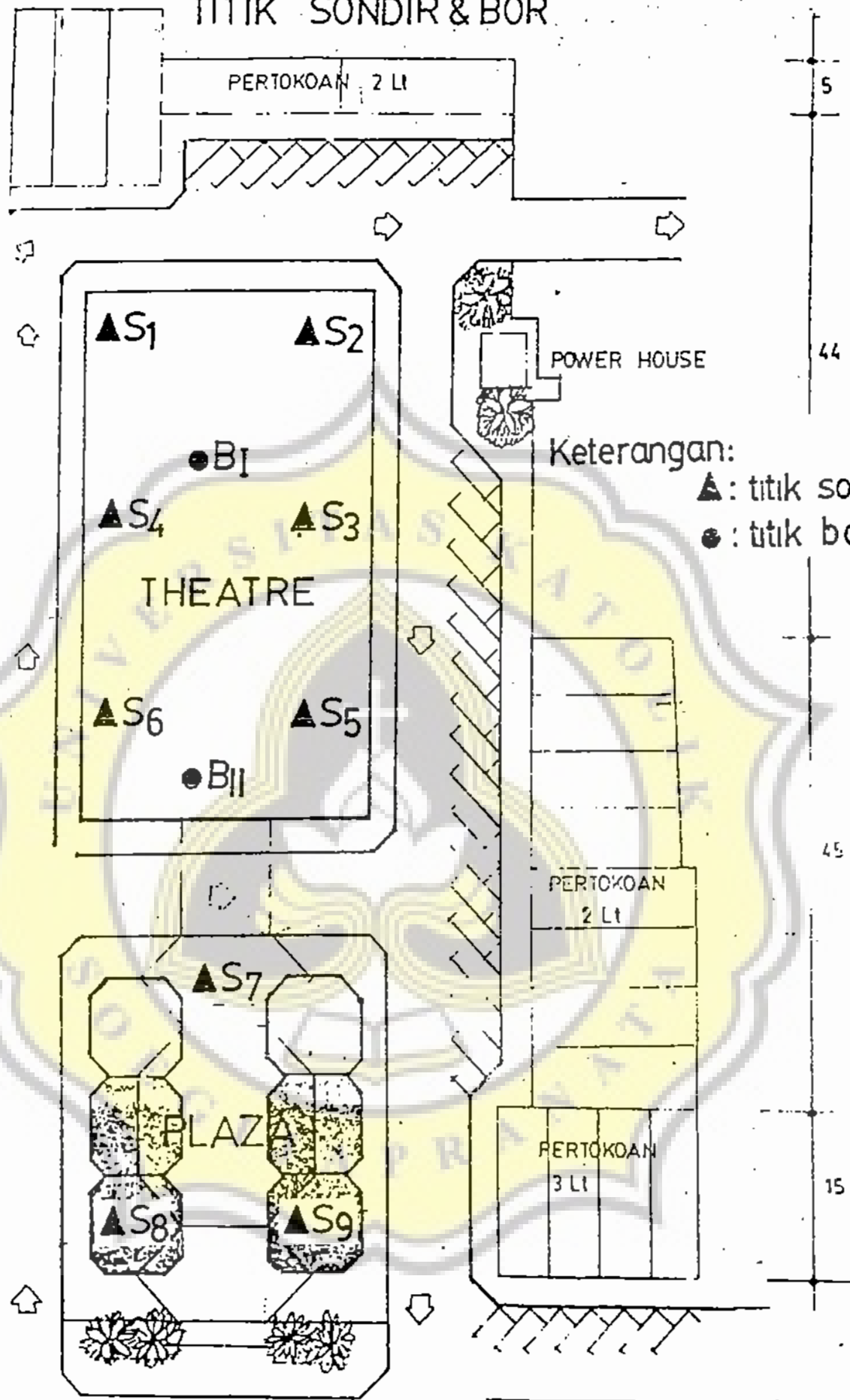
Fakultas Teknik U N D I P



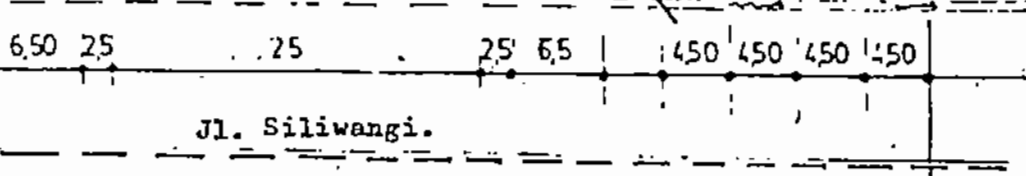
uvanto Budiharso .



SITUASI TITIK SONDIR & BOR



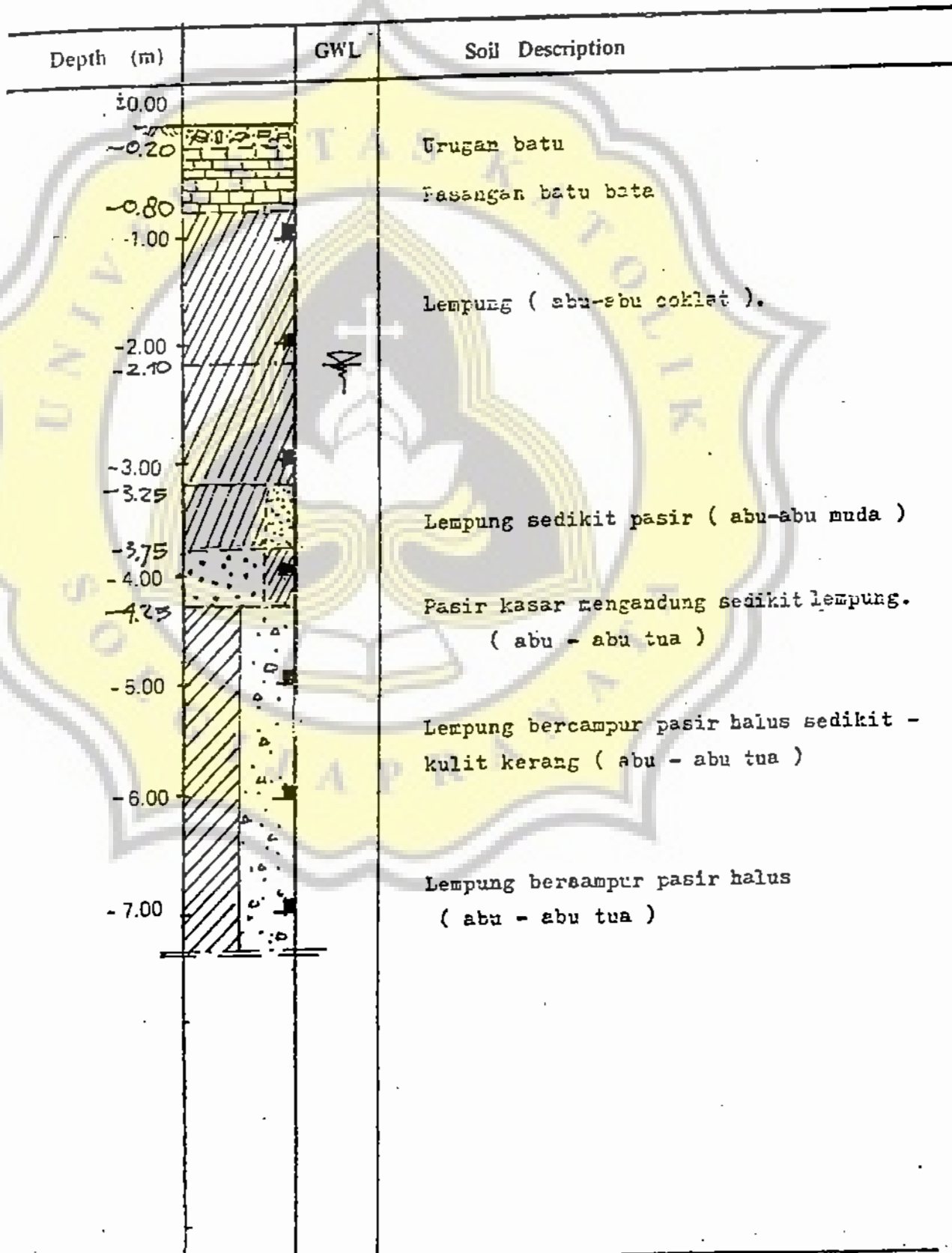
Keterangan:
 ▲ : titik sondir
 ● : titik bor



SOIL PROFIL

Project : P.T. Siliwengi Plaza
 Location : Jalan Jendral Sudirman Semarang.
 Sample Code : B.I

Date : 16 September 1983.



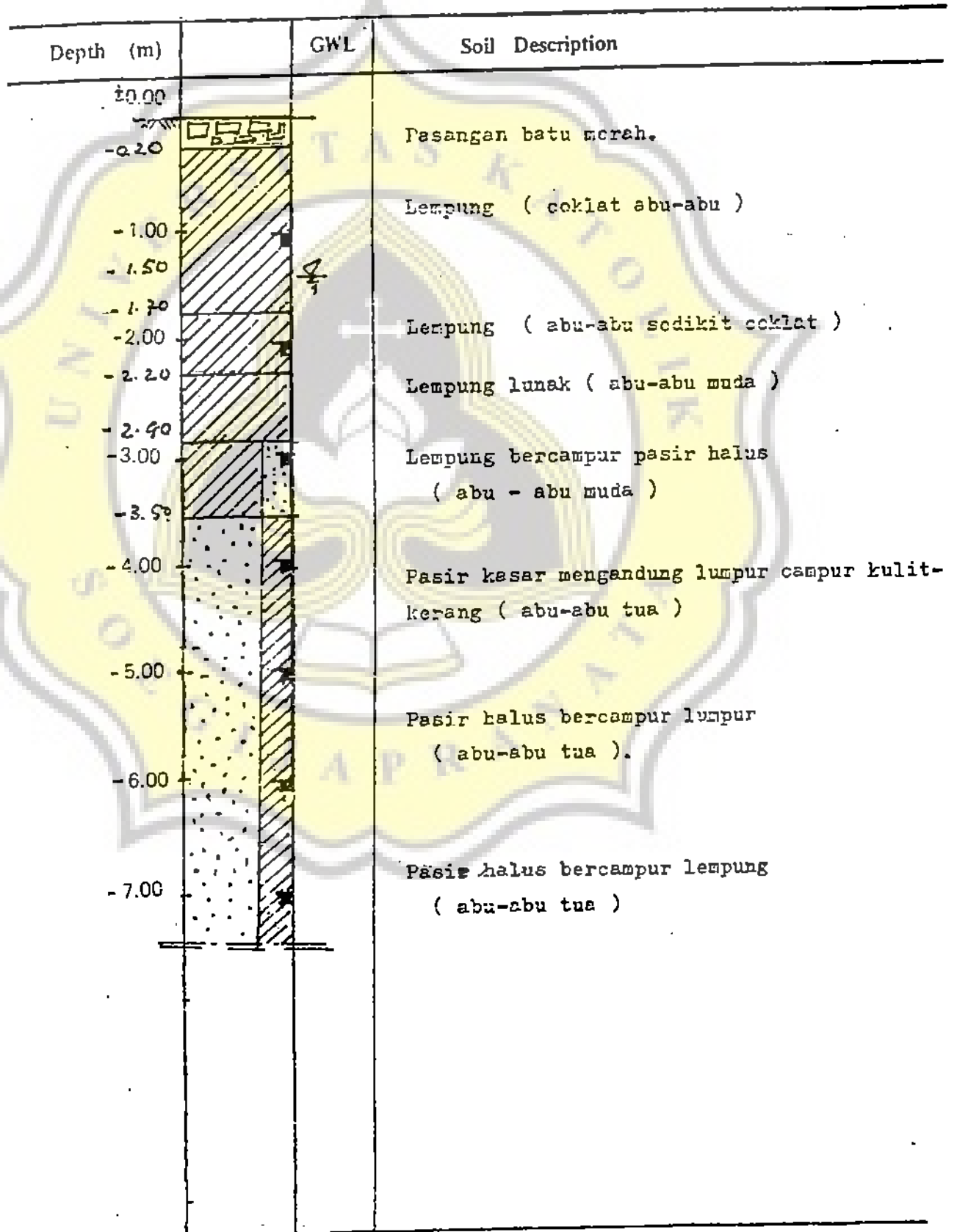
SOIL PROFIL

Project : P.T. Siliwangi Plaza

Location : Jalan Jendral Sudirman - Semarang.

Sample Code : B.II.

Date : 16 September 1983.



Soil Mechanics Laboratory Technical Faculty Diponegoro University			SOIL TEST							Location: SEMARANG PT. SILLIMANGI PLAZA	
No:	Boring No:	Sample Code	Depth (m)	Water Content (W) %	Specific gravity of solid (G _s)	Bulk density (ρ _b) gr/cm ³	Density (ρ _d) gr/cm ³	Wet density (ρ _w) gr/cm ³	Porosity (n) %	Void ratio (e)	Coefficient of permeability (k) cm/sec.
1	I	B I	1.00	40,57	2,5648	1,7669	1,2569	-	50,99	1,0405	-
2			2.00	55,83	2,5945	1,6512	1,0596	-	59,16	1,4486	-
3			3.00	58,07	2,6521	1,6846	1,0657	-	59,82	1,5395	-
4			4.00	26,04	2,9830	2,1159	1,6788	-	43,72	0,7769	-
5			5.00	49,44	2,7667	1,7461	1,1684	-	57,77	1,3634	-
6			6.00	40,23	2,8322	1,8564	1,3238	-	53,26	1,1392	-
7			7.00	40,69	2,7752	1,8337	1,2034	-	53,03	1,1250	-
8	II	B II	1.00	40,76	2,2923	1,6631	1,1850	-	48,30	0,9846	-
9			2.00	58,49	2,6451	1,6458	1,0384	-	60,74	1,5474	-
10			3.00	29,77	2,7969	1,9807	1,5263	-	45,43	0,8329	-

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Faculty of Engineering
Diponegoro University

GRAPH OF SOUNDING

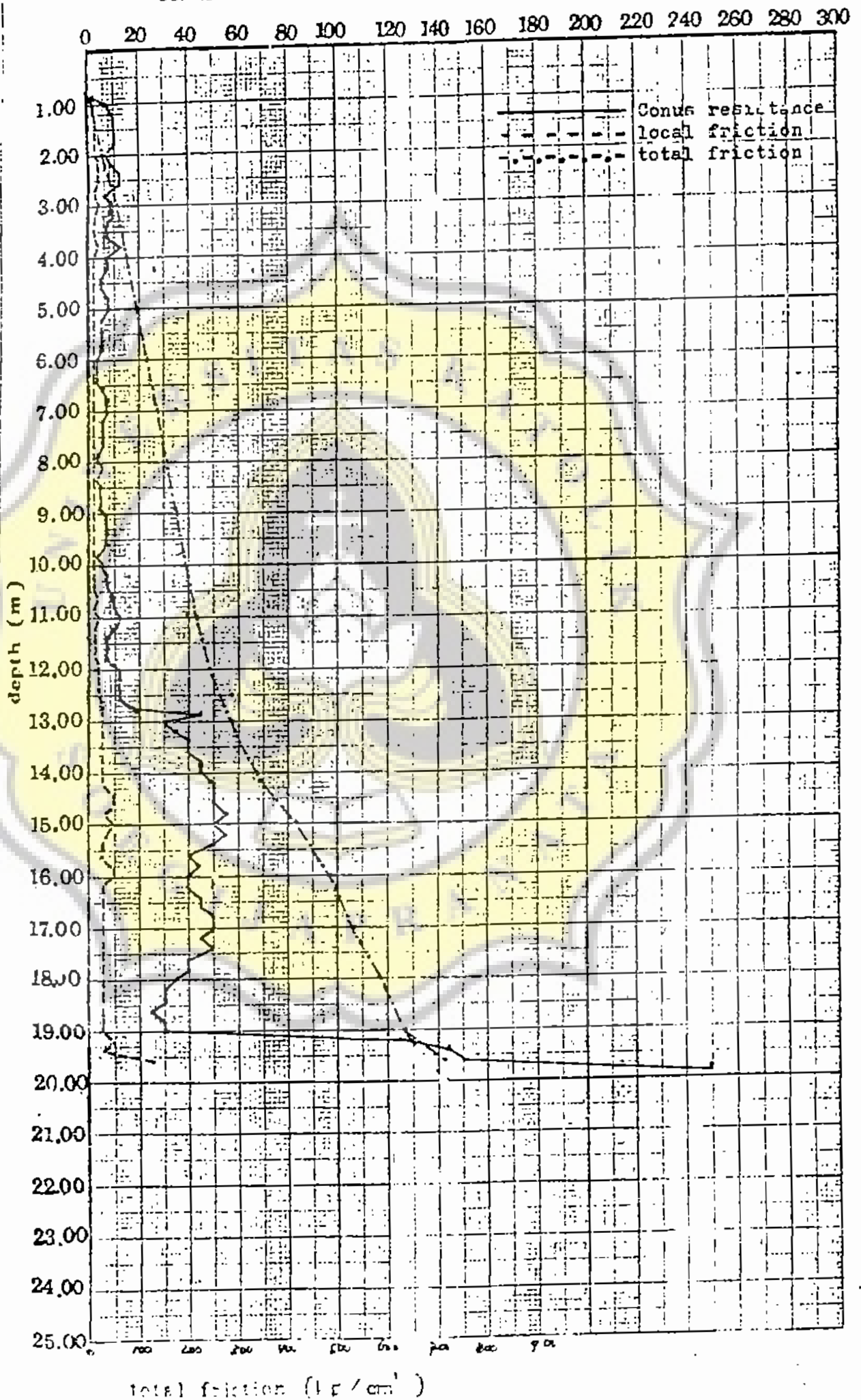
2,5 ton

Sounding No.: V

Location: PT. Siliwangi Plaza
Semarang.

Date : 18-9-1983

conus resistance & local friction (kg/cm^2)



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GRAPH OF SOUNDING

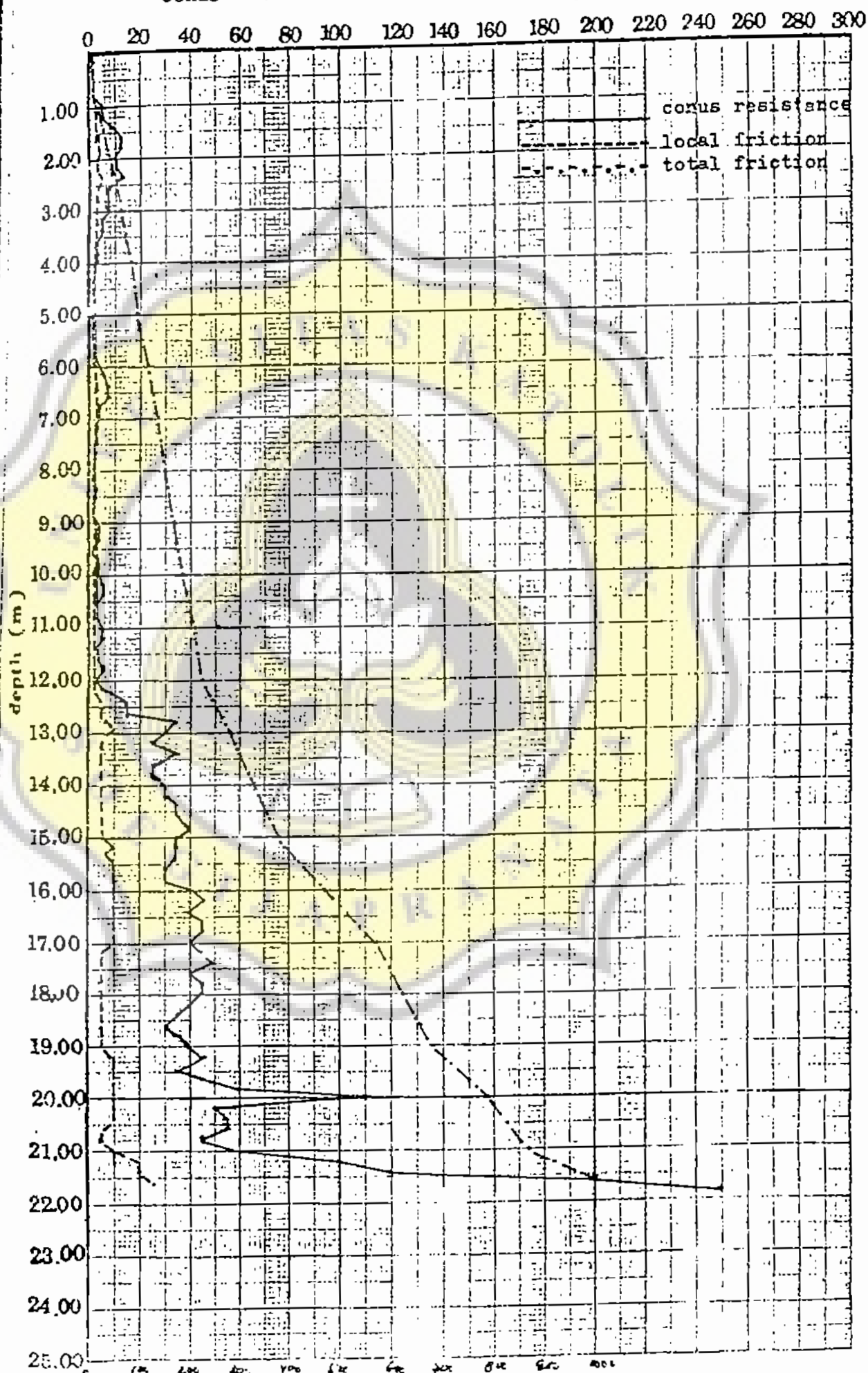
2,5 ton

Sounding No.: VI

Location: P.T. Siliwangi Plaza
Semarang.

Date : 18 - 9 - 1983.

conus resistance & local friction (kg/cm^2)



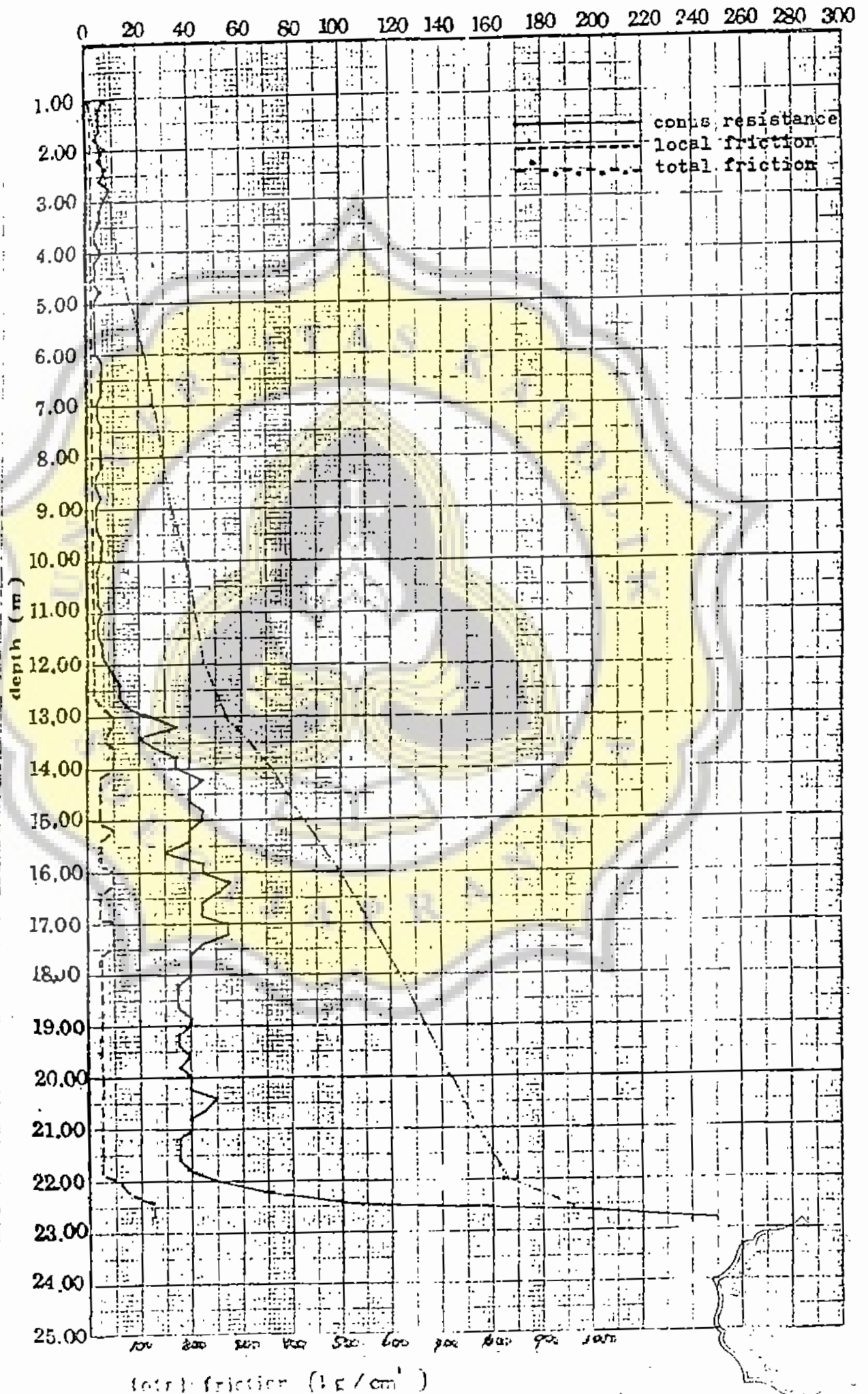
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GRAPH OF SOUNDING

2,5 ton

Sounding No.: VII
Location: P.T. Siliwangi Plaza
Semarang.
Date : 19 - 9 - 1983

conus resistance & local friction (kg/cm^2)



PER...

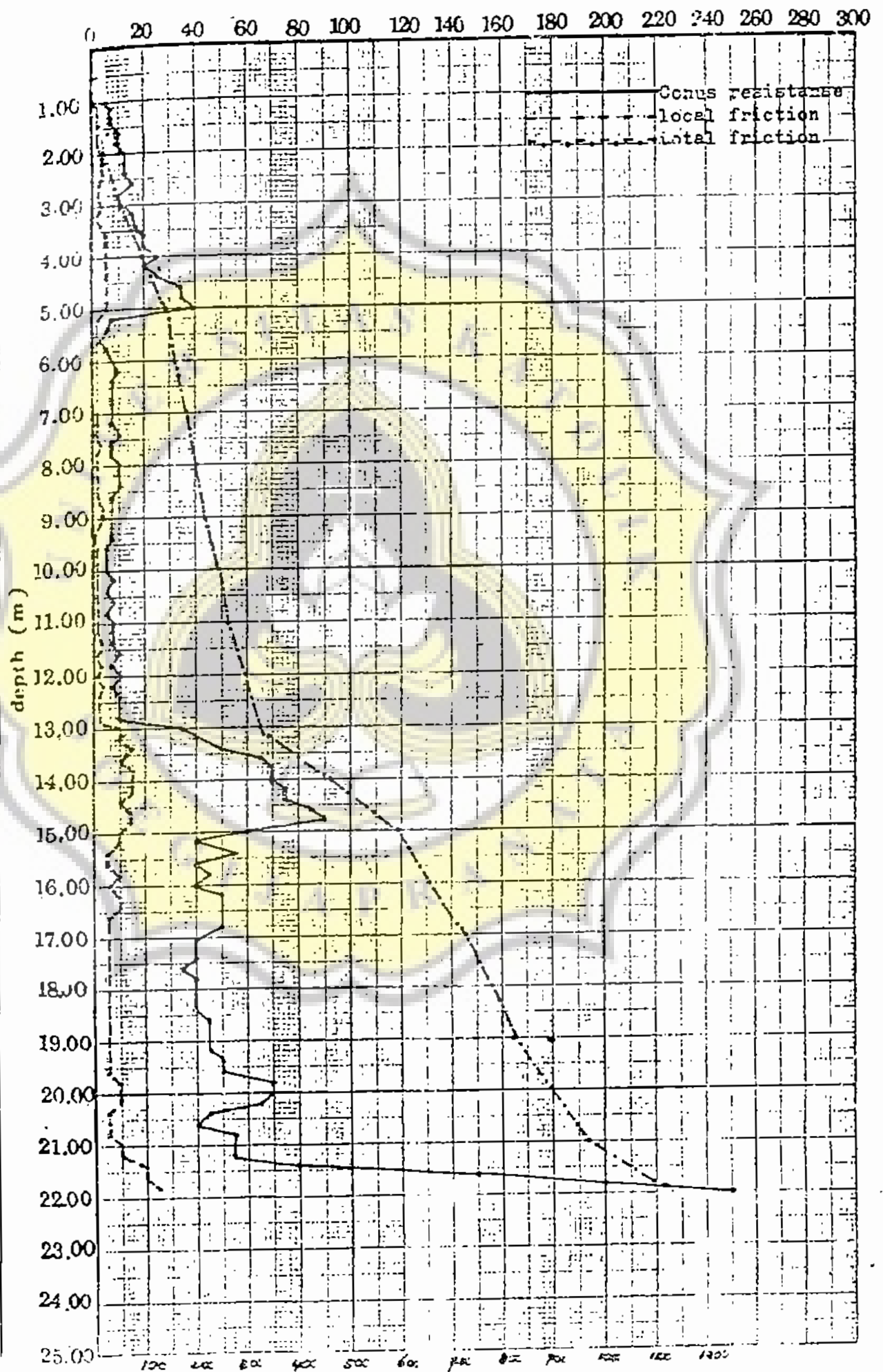
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GRAPH OF SCUNDING

2,5 ton

Sounding No.: I
Location: Siliwangi Plaza
Semarang.
Date : 16-9-1983

conus resistance & local friction. (kg/cm^2)



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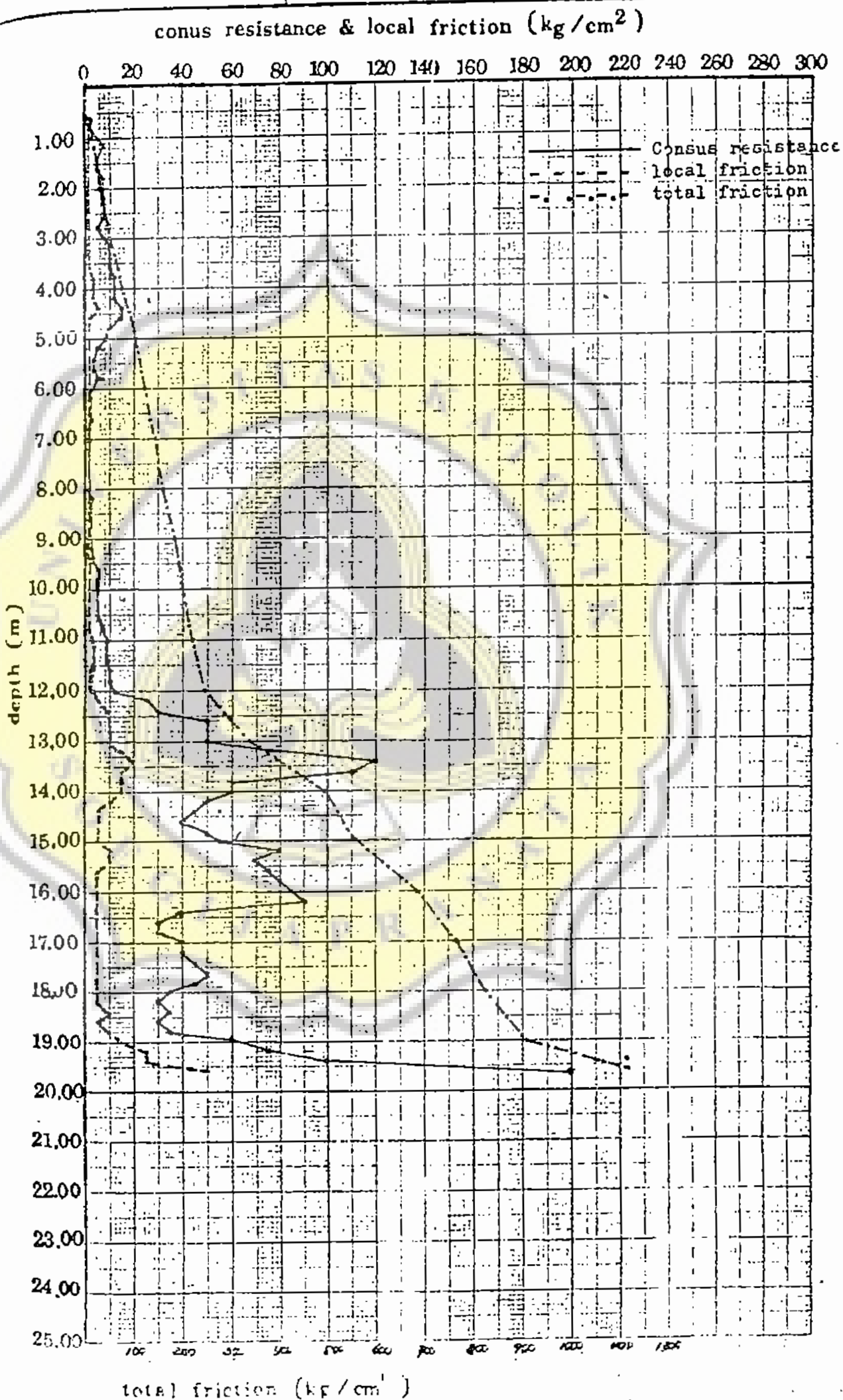
GRAPH OF SOUNDING

2,5 ton

Sounding No.: II.

Location: Siliwangi Plaza
Semarang.

Date : 16-9-1985



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GRAPH OF SOUNDING

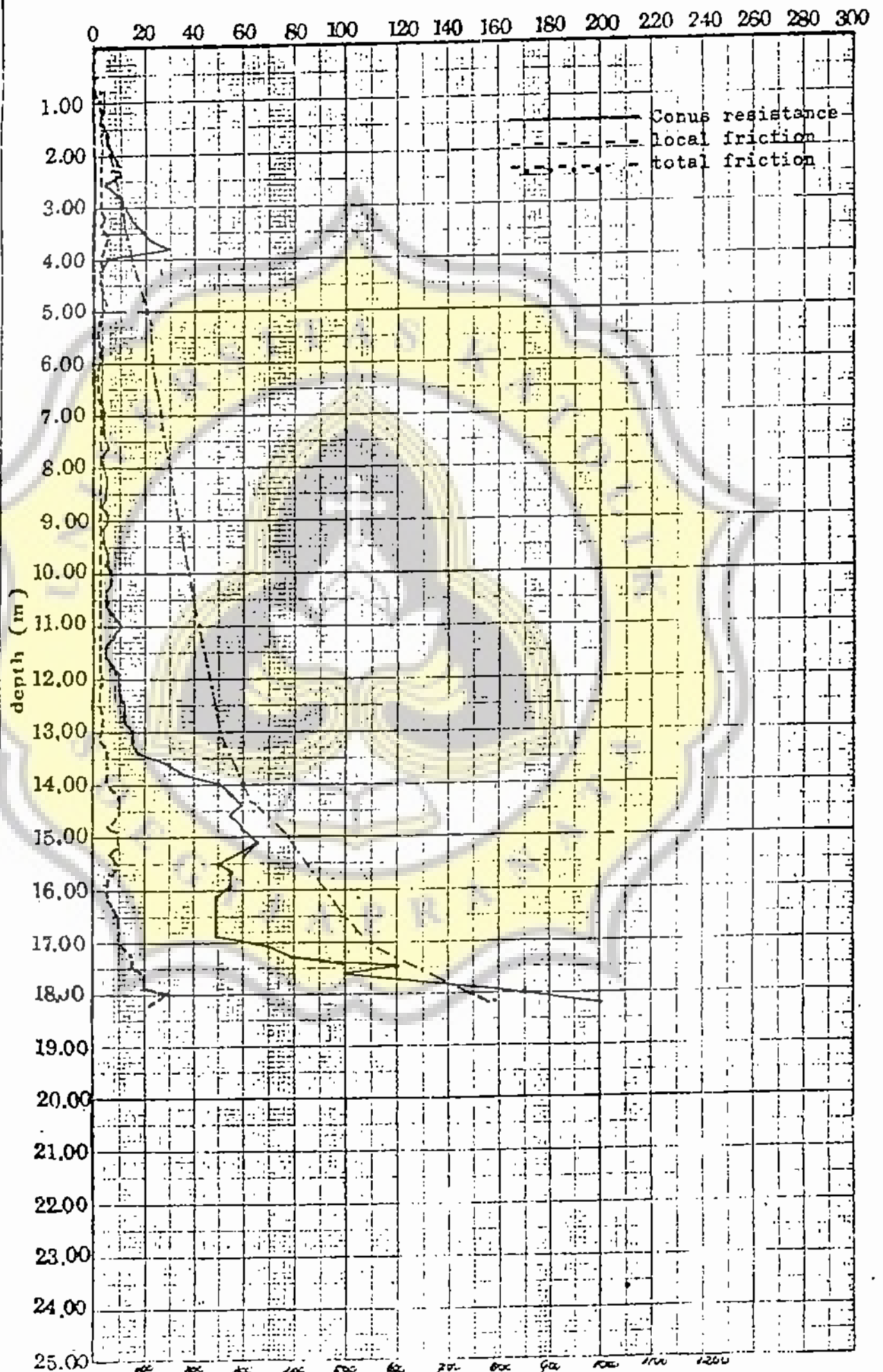
2,5 ton

Sounding No.: III

Location: PT.Siliwangi Plaza
Semarang.

Date : 17-9-1983

conus resistance & local friction (kg/cm^2)

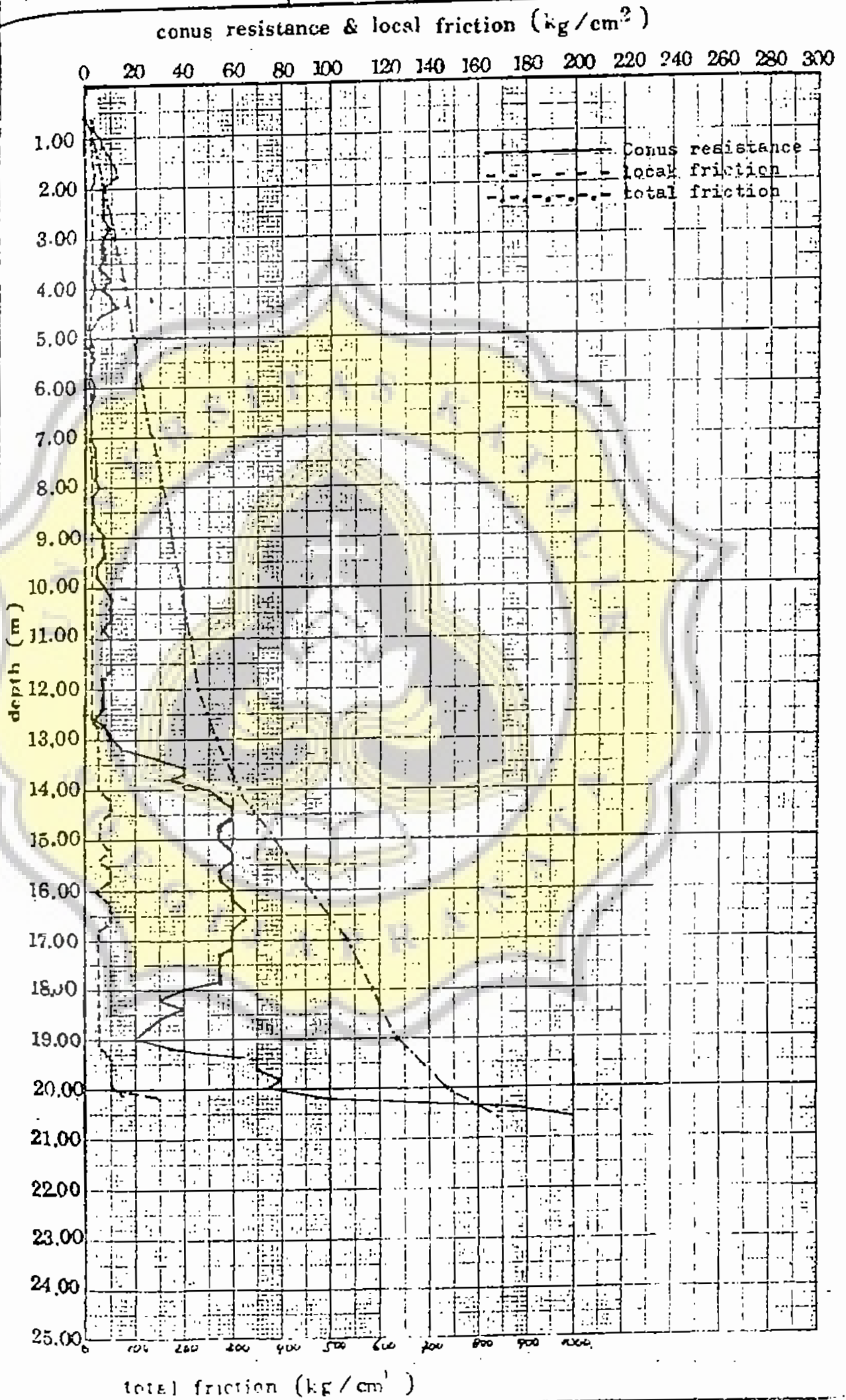


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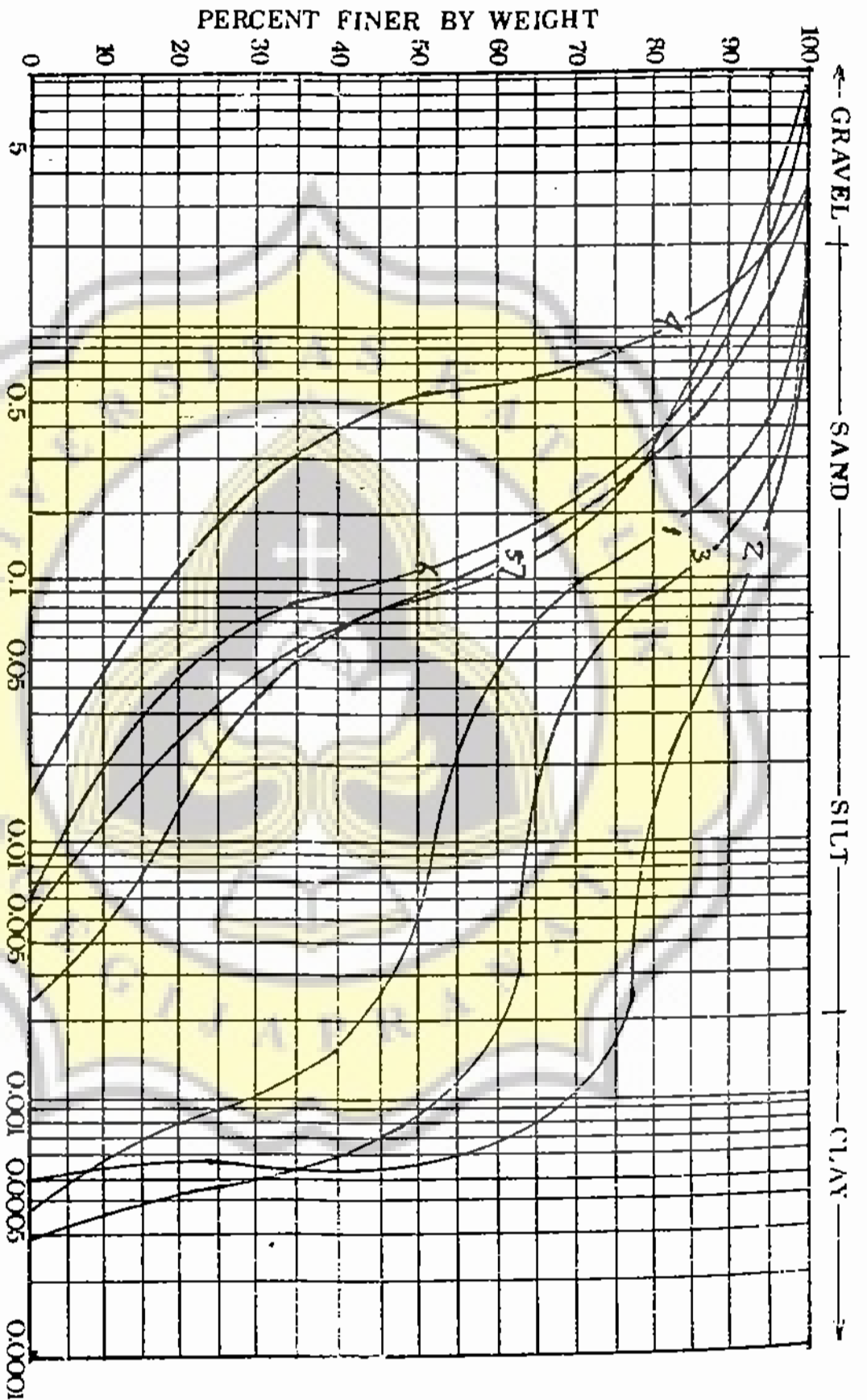
GRAPH OF SOUNDING

2.5 ton

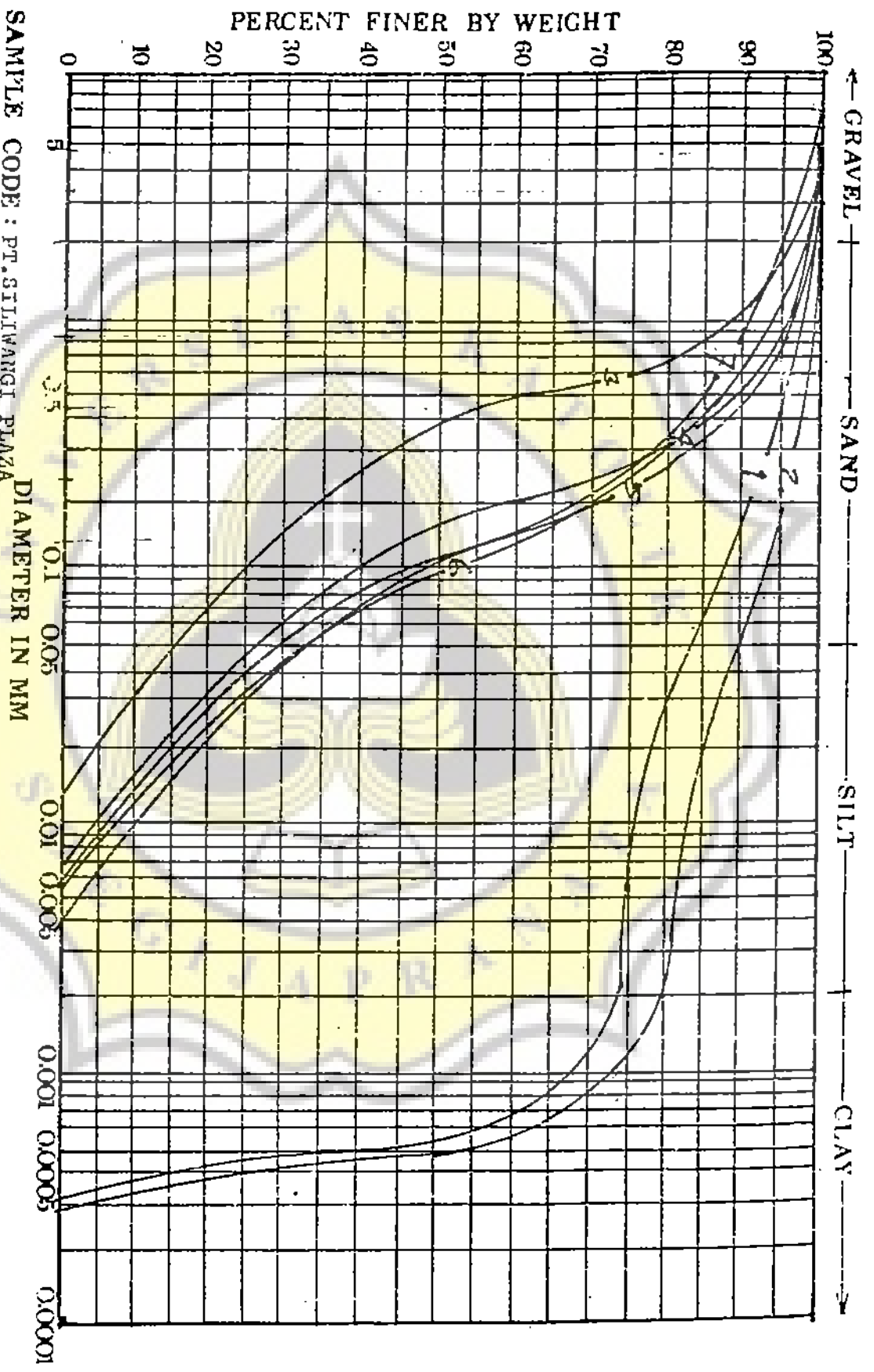
Sounding No.: IV
Location: TP. Sikiwangi Plaza
Semarang .
Date : 17-9-1983



GRAIN SIZE ACCUMULATION CURVE



GRAIN SIZE ACCUMULATION CURVE



SAMPLE CODE : PT. SITI WANGI PLAZA

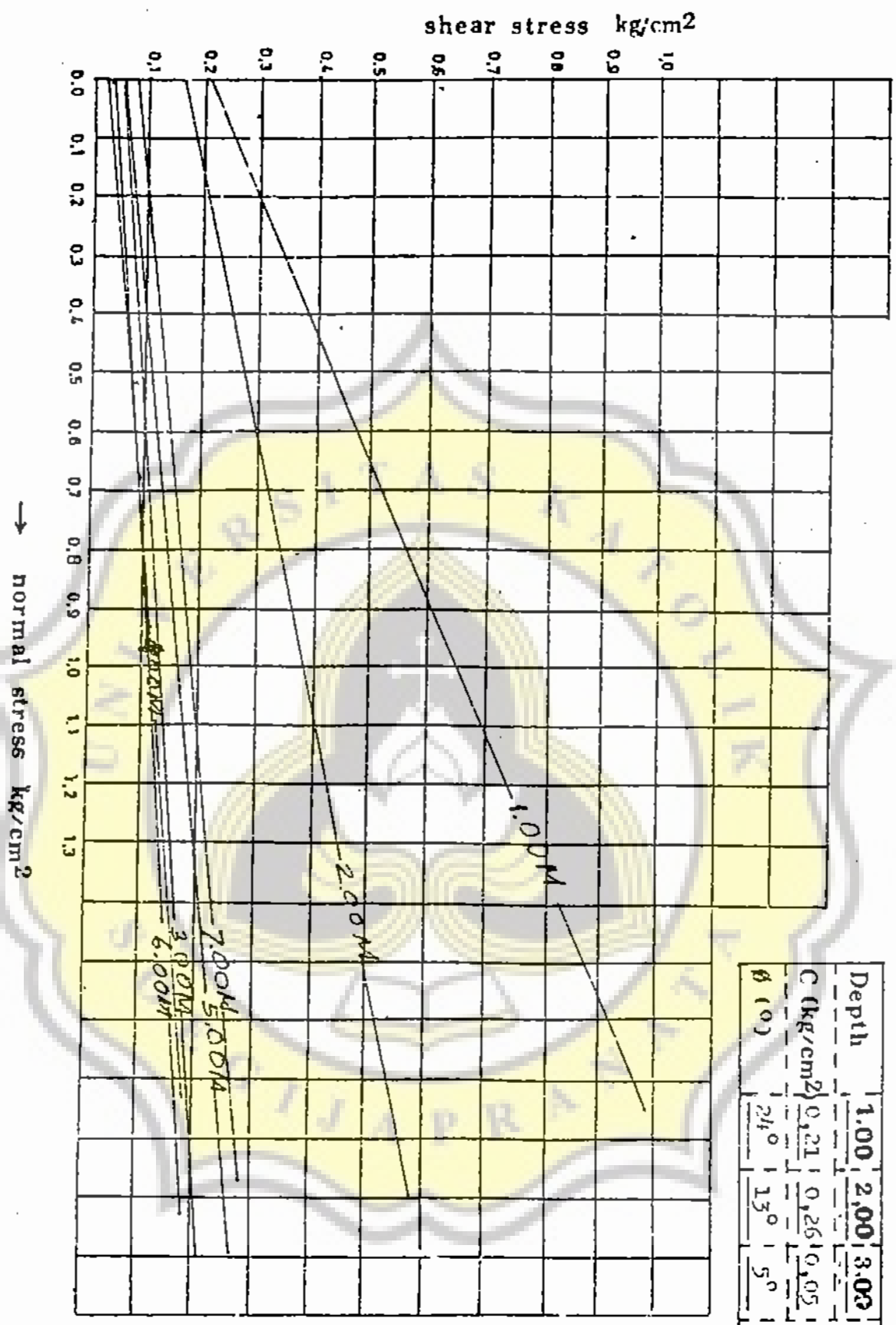
DIAMETER IN MM

LABORATORIUM MEKANIKA TANAH
 FAKULTAS TEKNIK BAGIAN SIPIL
 UNIVERSITAS DIPONEGORO

- 1 = RII.1.00
- 2 = RII.2.00
- 3 = RII.3.00
- 4 = RII.4.00
- 5 = RII.5.00
- 6 = RII.6.00

DIRECT SHEAR TESTS

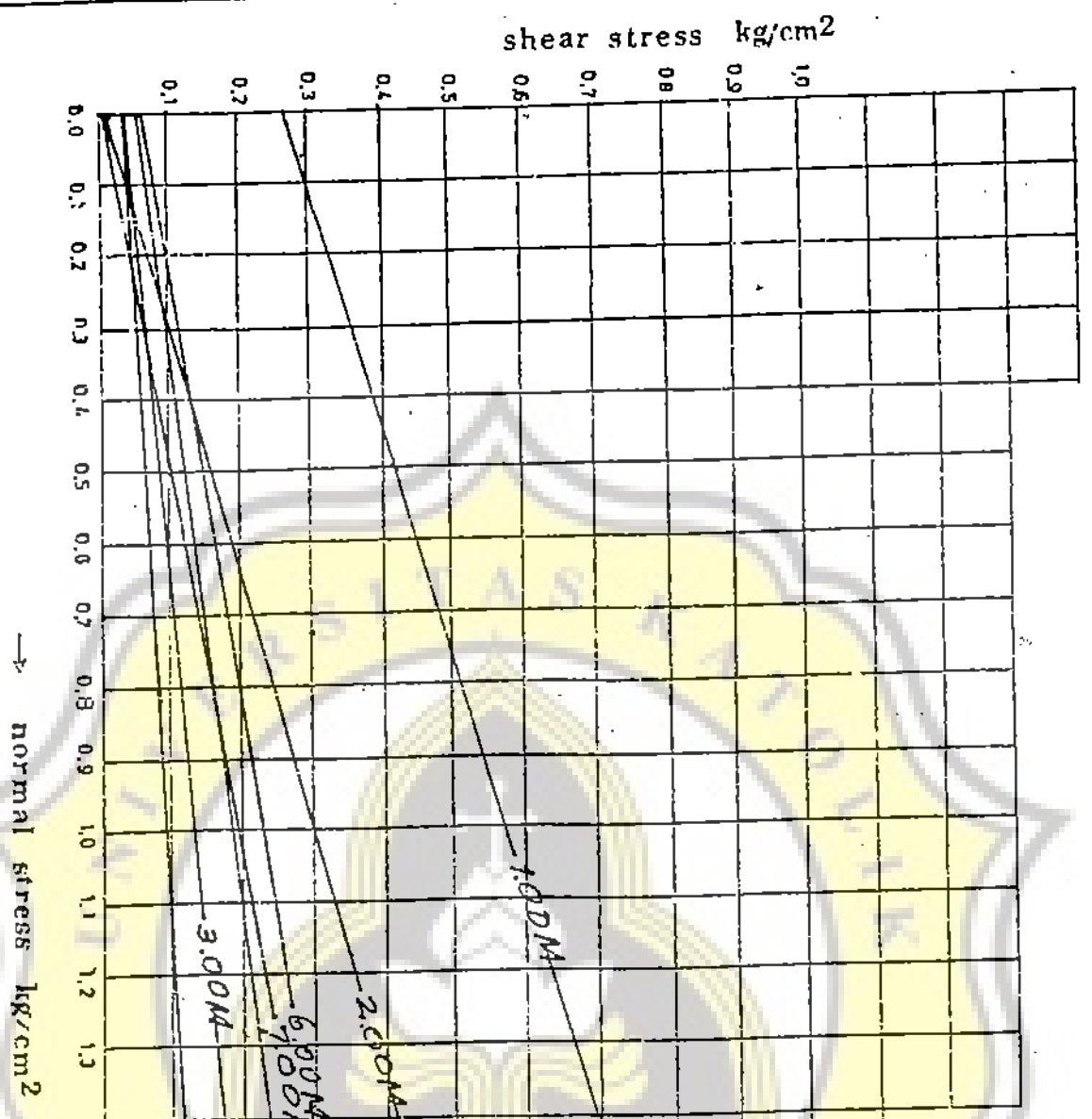
Report No. 23
 Project E.M. Laboratory 1983
 Location Semarang



Depth	1.00	2.00	3.00	4.00	5.00	6.00	7.00
C (kg/cm ²)	0.21	0.26	0.05	0.01	0.05	0.02	0.07
ϕ (°)	24°	13°	5°	35°	5°	6°	5°

DIRECT SHEAR TESTS

Room No. B.11.
 Project U.P. of Advanced Phase
 Location Semarang



Depth	1.00	2.00	3.00	4.00	5.00	6.00	7.00
C (kg/cm ²)	0.26	0.02	0.03	0.04	0.03	0.05	0.012
ϕ (°)	17°	15°	5°	9°	5°	11°	10°

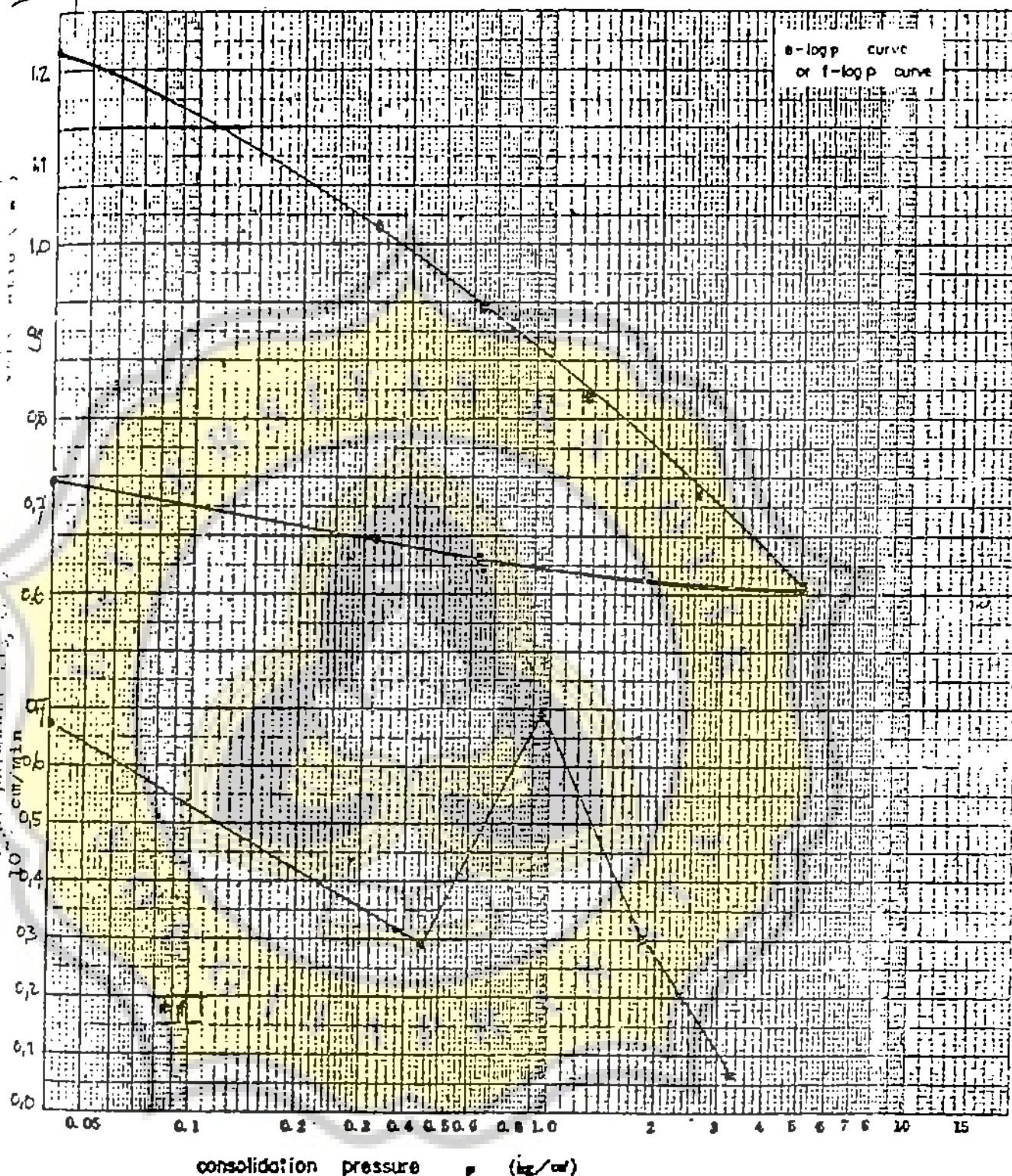
CONSOLIDATION TEST

PROJECT BILIWANGI PLAZA DATE OF TESTING _____
 LOCATION SEMARANG TESTED BY _____
 SAMPLE NO. B.1 Depth -3.10

Soil condition		Room temperature (°C - °C)			Tester No.	No.				
Nature of soil	Soil classification	-	Specimen	Section area A cm ²	31.156	Initial water content w ₀ %	58.07			
	Soil gravity G _s	2.6521		Initial height h ₀ cm	2.00	Initial volume ratio f ₀	2.2212			
	Liquid limit w _L %	-		Dry weight W _d g	74.40	Initial void ratio e ₀	1.5395			
	Plastic limit w _p %	-		Height of substance h _s cm	0.9004	Initial degree of saturation S _{rv} %	-			
Load	Pressure	Consolidation volume	Height of specimen	Average height of specimen	Compression strain	Coefficient of volume compression	Volume ratio	Void ratio	Calculation formulae	
Step	P kg/cm ²	ΔV cm ³	Δ (10 ⁻² cm)	A =	Δe %	C _v / 10 ⁻²	f	e		
0	0.00		2.00	1.9093	9.50	29.607	2.2212	1.2212	$A_c = \frac{W_d}{G_s \cdot V_s}$	
1	0.32	78.40	1.8166	1.7795	4.383	13.677	2.0128	1.0128	$\Delta e = \frac{\Delta d}{d}$	
2	0.64	78.0	1.7407	1.6916	5.789	17.045	1.9331	0.9331	$u = \frac{d_e (5\%) \cdot 1}{\Delta p} \cdot 100$	
3	1.28	97.90	1.6427	1.5939	6.87	21.42	1.8244	0.8244	$f = \frac{A_c}{A}$	
4	2.57	102.80	1.5452	1.4632	9.596	3.741	1.7167	0.7167	$e = f - 1$	
5	5.135	-41.70	1.4424	1.4032	-2.849	-0.6347	1.6019	0.6019	$S_{rv} = \frac{G_s \cdot w}{e}$	
6	0.64	0.32	-15.0	1.4416	-1.006	-3.1437	1.6483	0.6483	$\bar{p} = \sqrt{p_u \cdot p_{u+1}}$	
7	0.32	0.32	-59.80	1.4491	1.5286	-3.866	1.6649	0.6649	\sqrt{t} Method	
8	0.00			1.5582			1.7306	0.7306	$C_c = \frac{0.197 \left(\frac{\Delta}{z}\right)^2}{e_m}$	
									Curve rule method	
									$C_c = \frac{\Delta e}{\Delta p} \cdot C$	
									$k = \frac{C_c \cdot a_v \cdot 17}{L \cdot 100}$	
Load	Pressure	$0.848 \left(\frac{A}{z}\right)^2$	$0.197 \left(\frac{A}{z}\right)^2$	t_{90} min	C_c	Initial consolidation volume $\frac{\Delta d}{\Delta d}$ (10 ⁻² cm)	Initial consolidation ratio $\frac{\Delta d}{\Delta d}$	Coefficient of consolidation C_v cm ² /min	Coefficient of permeability k cm/min	Remarks:
Step	P kg/cm ²	P kg/cm ²	P kg/cm ²	t ₅₀ min	C _v / min	(10 ⁻² cm)	$\frac{\Delta d}{\Delta d}$	cm ² /min	cm/min	m-5.
0	0.00			30.25	0.1255	161.10	0.8881	0.0226	0.6723	
1	0.32	0.453	0.6714	16.61	0.2404	41.10	0.5209	0.0213	0.2917	
2	0.64	0.905	0.6055	2.89	0.2099	35.00	0.3636	0.0763	0.1004	
3	1.28	1.814	0.5386	4.203	0.1281	47.00	0.4493	0.0628	0.2980	
4	2.57	3.633	0.2432	7.89	0.0334	48.30	0.4757	0.0158	0.0594	
5	5.135									
6	0.64									
7	0.32									
8	0.00									
CALCULATION DATA										
Md.										
Dr.										
Ch.										
Job No.										

CONSOLIDATION TEST

PROJECT SILIWANGI PLAZA DATE OF TESTING _____
 LOCATION SEMARANG. TESTED BY _____
 BORING NO. I - 3.00



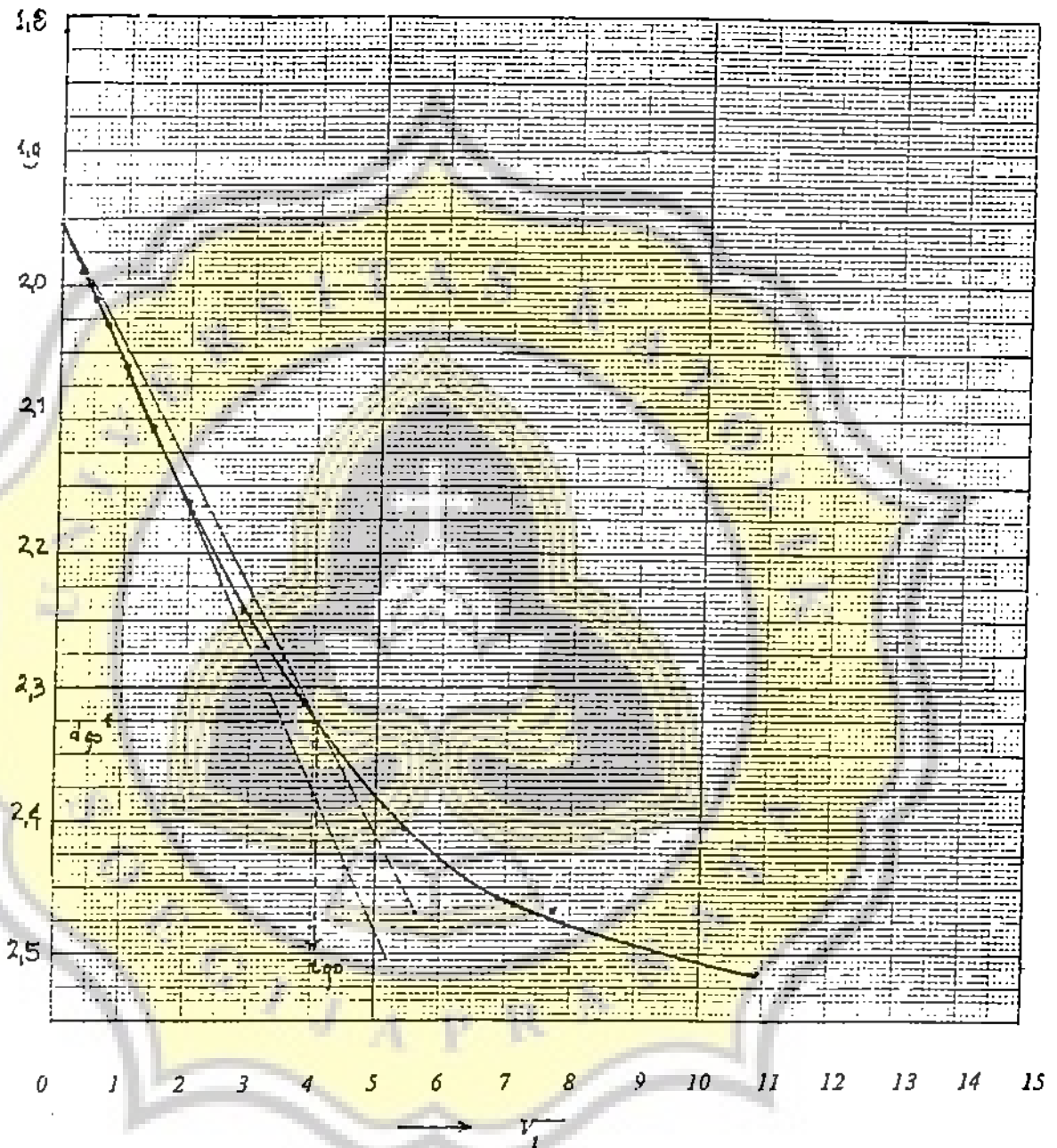
Sample No.	Depth	Liquid limit w _L %	Initial volume ratio I _v	Initial void ratio	Coefficient of compression C _c	Vertical stress of consolidation p _v (kg/cm ²)	Stcr.
B1	- 3.00 m	-	2.2212	1.5395	1.2866		
					0.4825		
					0.6486		

	Md.		Job No.
	Dr.		
e- or i- log p REPORTS	Ch.		

CONSOLIDATION TEST

(V_f Curve)

Project STLIWANGI PLAZA
 Location SEMARANG
 Sample No. E.I Depth -5.00 Date of testing _____
 Soil Description _____ Tested by _____



Load Step	2 kg		
Consolidation pressure kg/cm ²	0,64		
Initial value (d _i)	1,8140		
Compensated initial value (d ₀)	1,9550		
d ₉₀	2,3250		
Final value d _f	2,5940		
i ₉₀	16,61		
$\Delta e = 10/9 (d_0 - d_{90})$	0,411		

CONSOLIDATION TEST

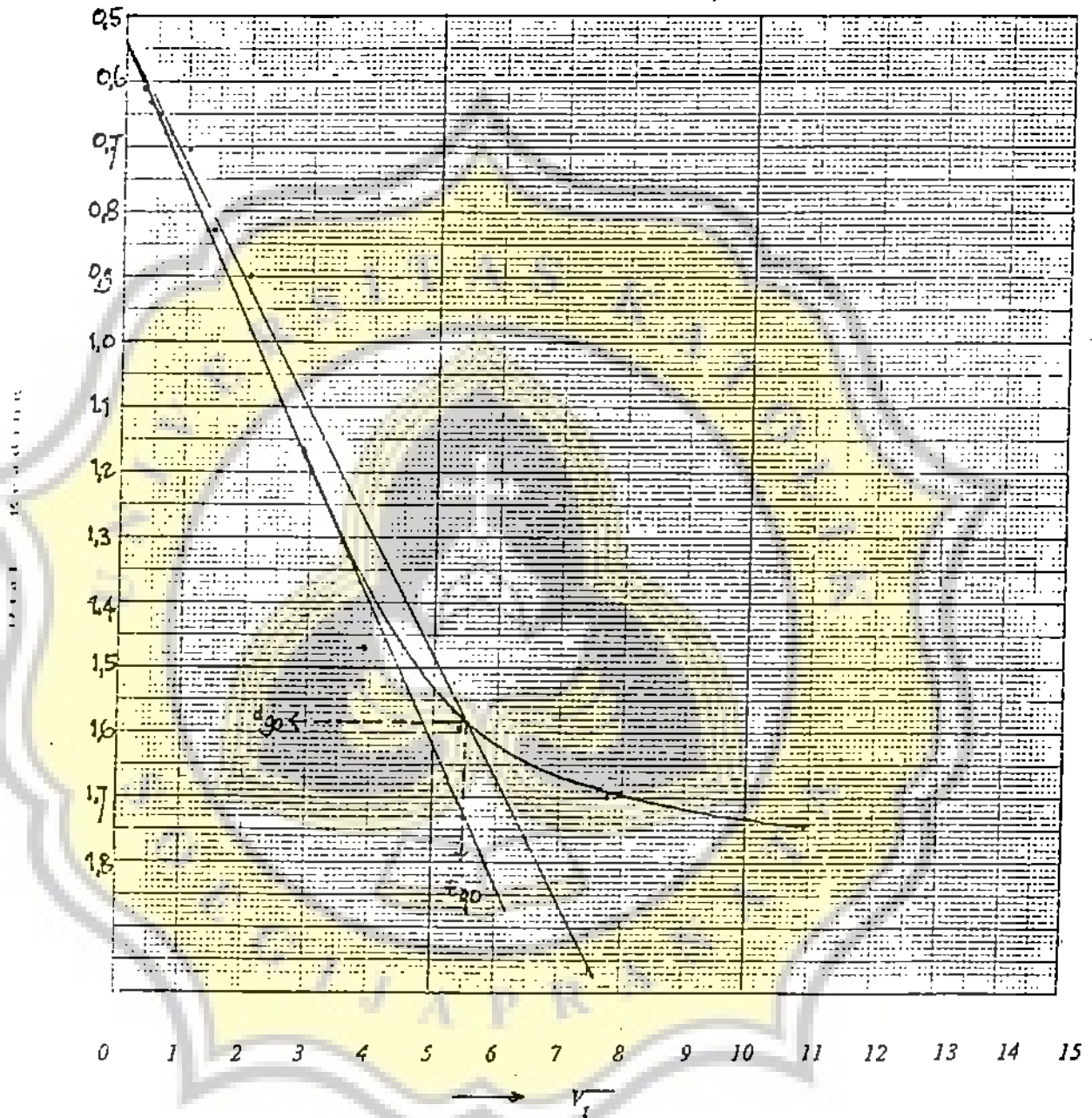
(V_t Curve)

Project SILIWANGI PLAZA

Location SEMARANG

Sample No. B.I Depth 1-3,00 Date of testing _____

Soil Description _____ Tested by _____



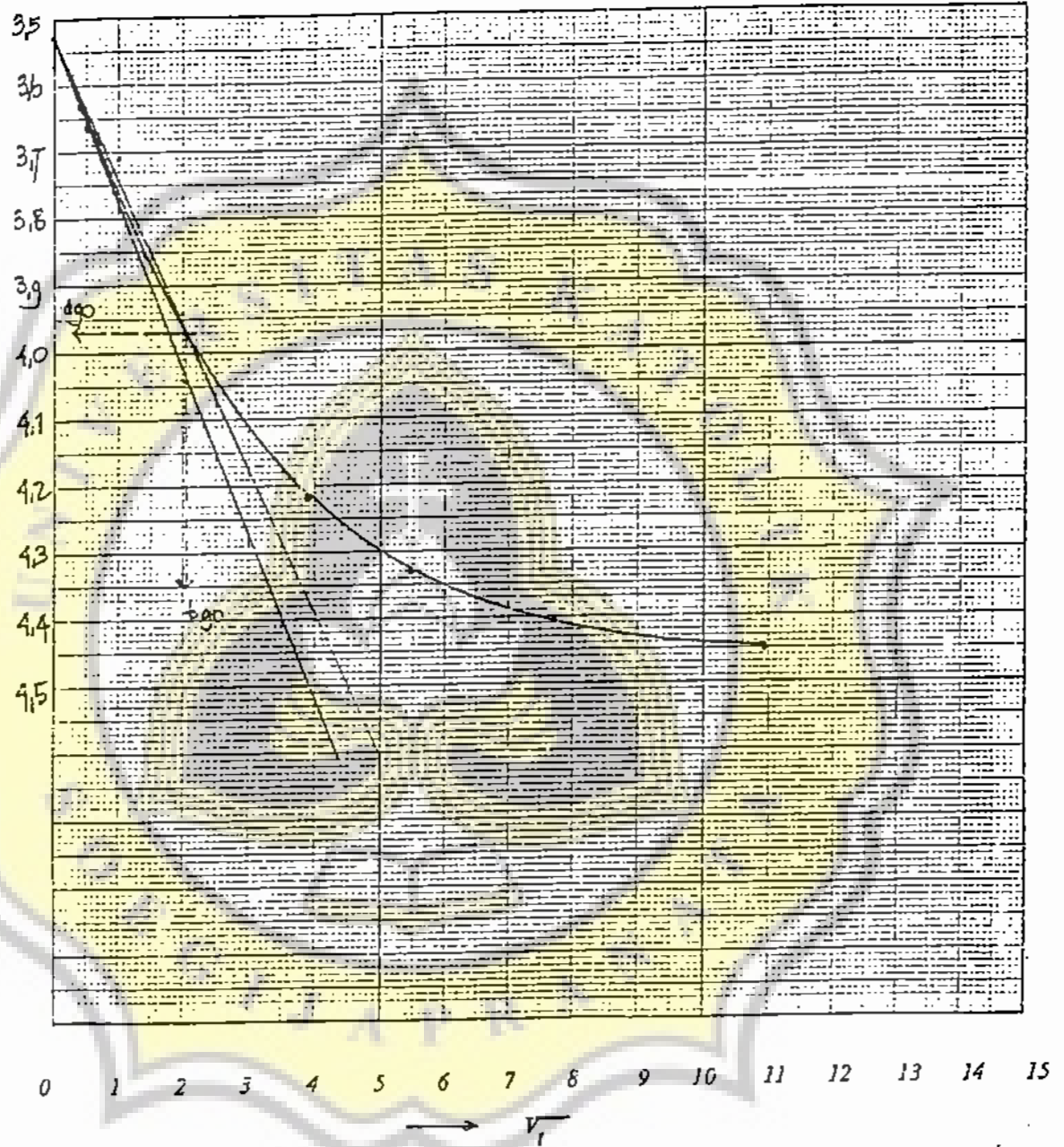
Load Step	1 kg		
Consolidation pressure kg/cm ²	0,32		
Initial value (di)	0,000		
Compensated initial value (d ₀)	0,540		
d 90	1,585		
Final value df	1,8140		
190	30,25		
$\Delta d^* = 10/9 (d_0 - d_{90})$	1,011		

PERANG

CONSOLIDATION TEST

(\sqrt{t} Curve)

Project SILIWANGI PLAZA
 Location SEMARANG
 Sample No. B.I Depth -3.00 Date of testing _____
 Soil Description _____ Tested by _____

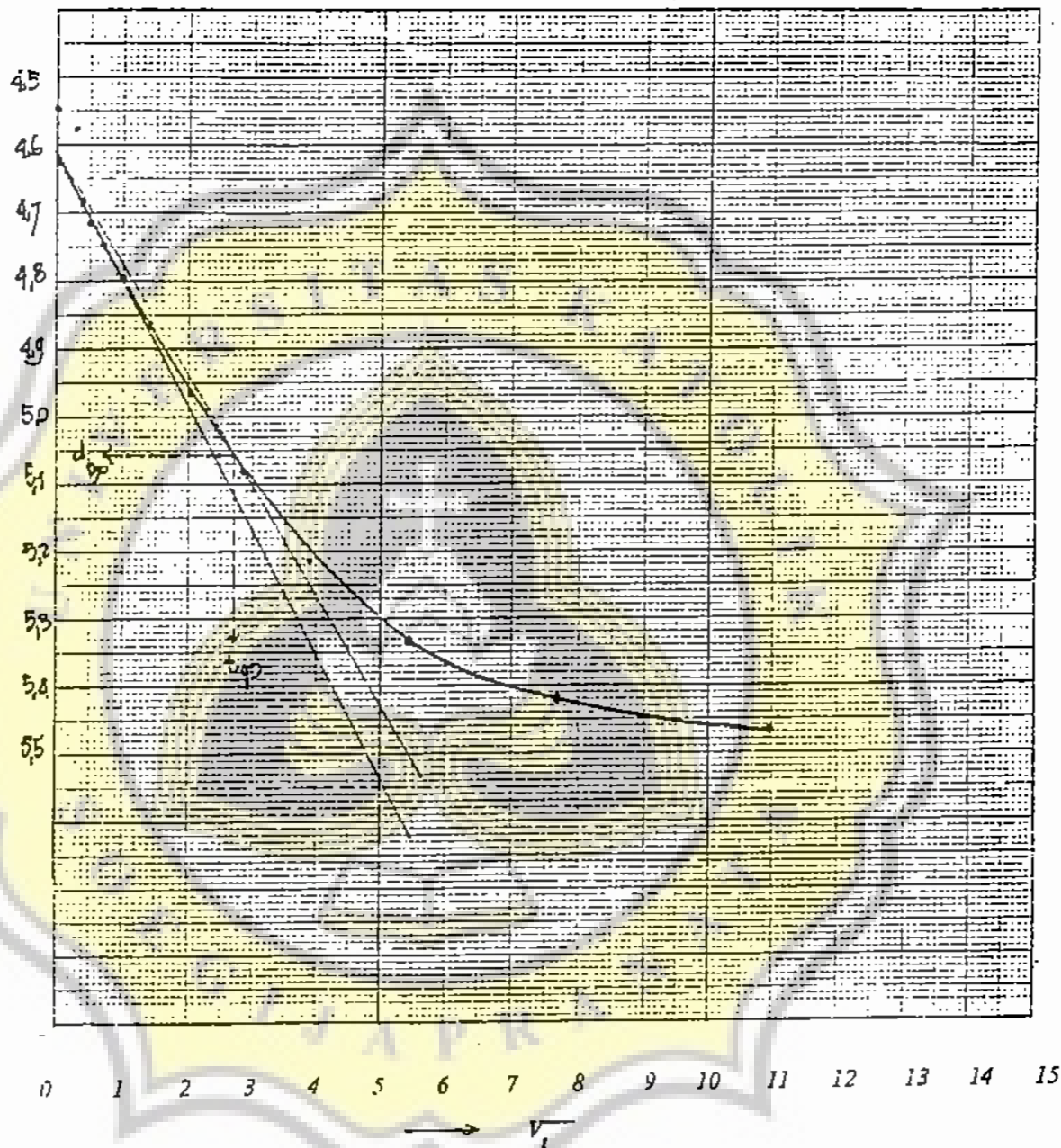


Load Step	8 kg		
Consolidation pressure kg/cm ²	2.57		
Initial value (d _i)	3.5730		
Compensated initial value (d ₀)	3.540		
d ₉₀	3.970		
Final value d _f	4.5480		
190	4.203		
$\Delta d' = 10/9 (d_0 - d_{90})$	0.478		

CONSOLIDATION TEST

(V_e Curve)

Project SILIWANGI PLAZA
 Location SEMARANG
 Sample No. B.I Depth -3.00 Date of testing _____
 Soil Description _____ Tested by _____

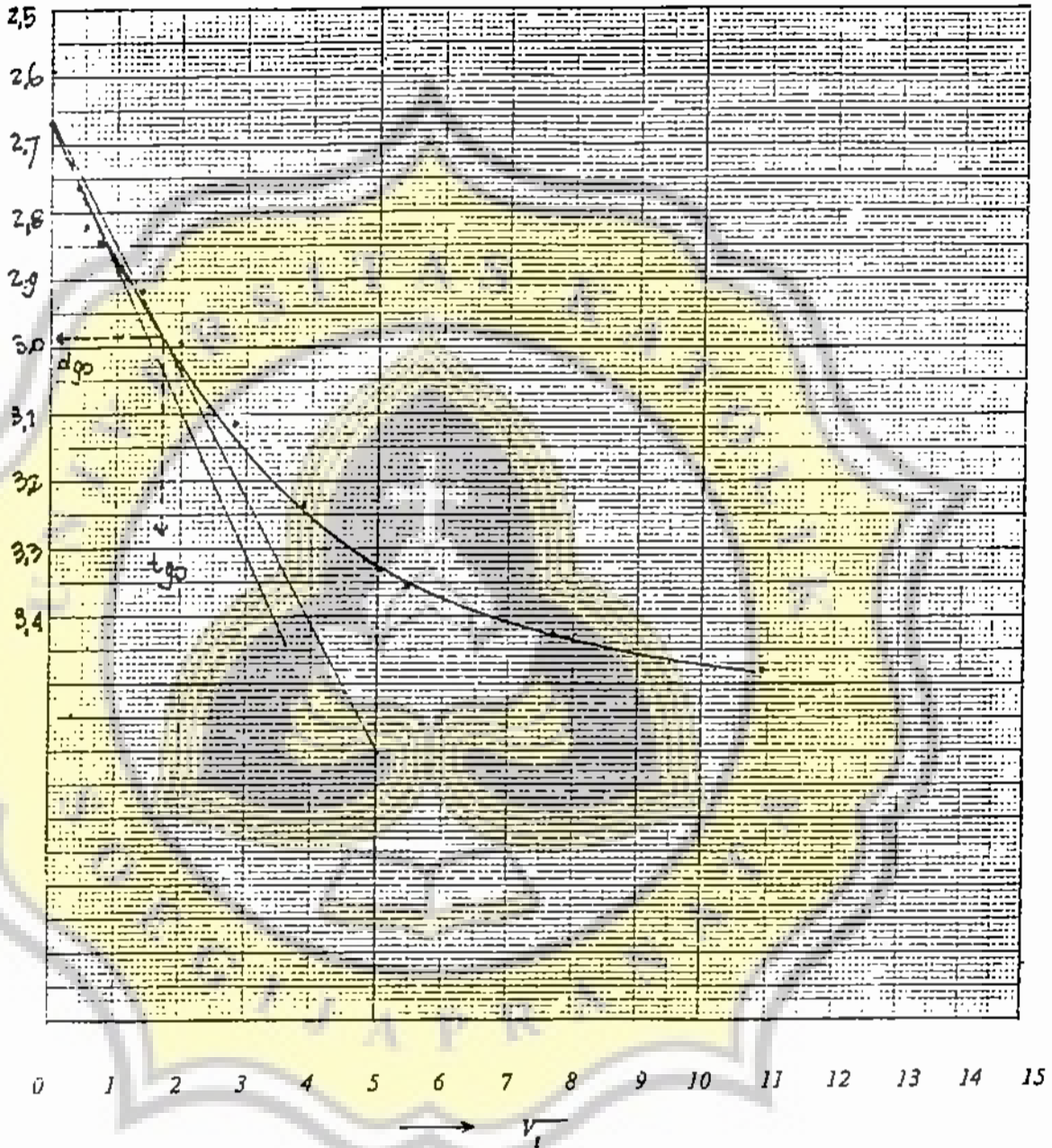


Load Step	1.6 kg		
Consolidation pressure kg/cm ²	5		
Initial value (di)	4,5480		
Compensated initial value (do)	4,6150		
d 90	5,0550		
Final value df	5,5760		
190	7,29		
$\Delta d' = 10/9 (d_{90} - d_{90})$	0,489		

CONSOLIDATION TEST

(V_v Curve)

Project SILIWANGI FLAZA
 Location SEMARANG
 Sample No. B.I Depth -3.00 Date of testing _____
 Soil Description _____ Tested by _____



Load Step	4 kg		
Consolidation pressure kg/cm ²	1,28		
Initial value (di)	2,5940		
Compensated initial value (do)	2,6650		
d 90	2,9850		
Final value df	3,5730		
190	2,890		
$\Delta d' = 10/9 (do - d90)$	0,356		

CONSOLIDATION TEST

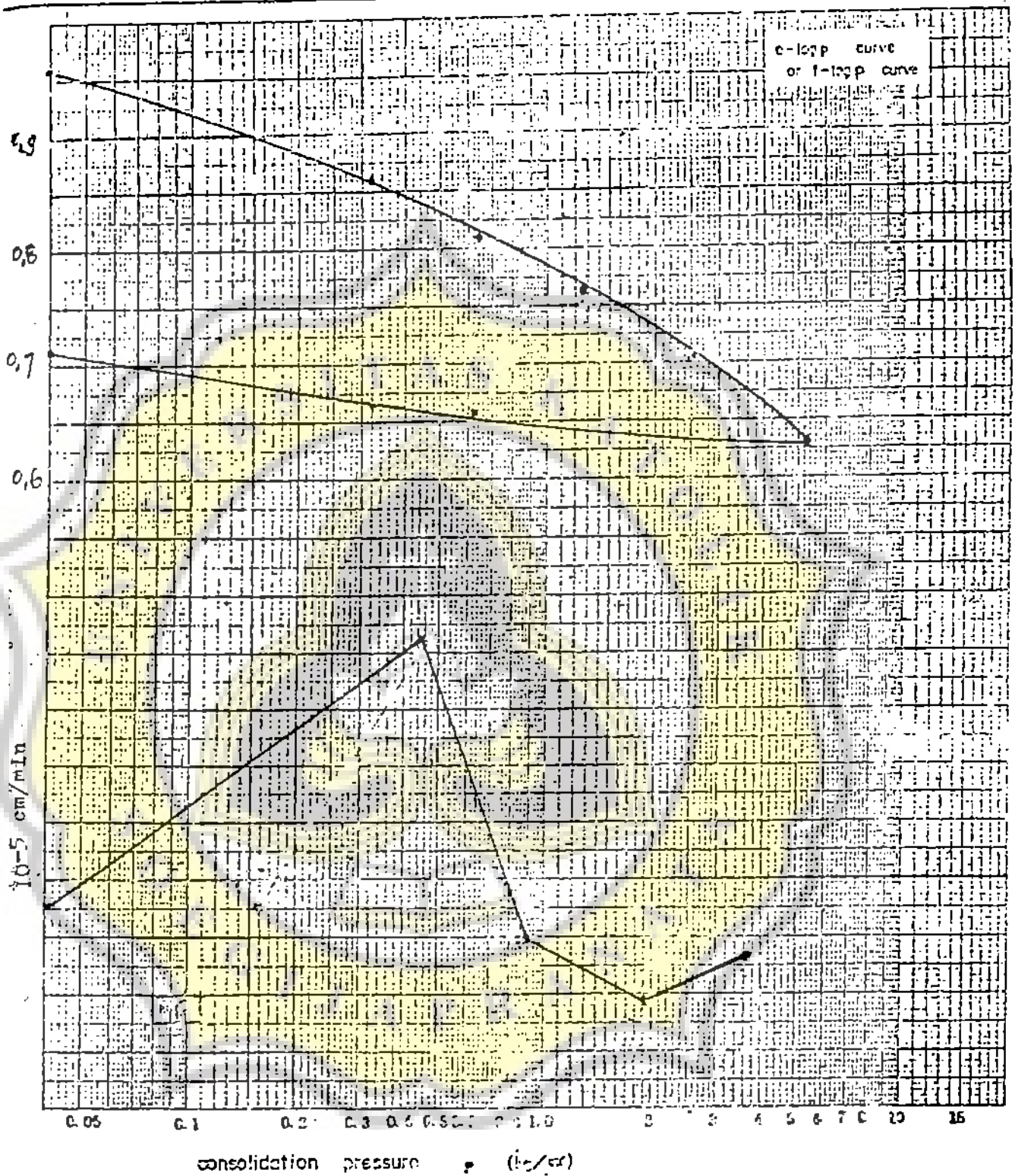
PROJECT SILIWANGI PLAZA DATE OF TESTING _____
 LOCATION SEMARANG TESTED BY _____
 SAMPLE NO. B.I. Depth -5.00

Sample condition		Room temperature ($t \sim t$)					Tester No.	No.	
Nature of soil	Soil classification						Initial water content w %		
	Soil gravity G_s	2.767					Initial volume ratio f_0		
	Liquid limit w_L %	-					Initial void ratio e_0		
	Plastic limit w_p %	-					Initial degree of saturation S_{r0} %		
			Section area	A cm^2	31.157				
			Initial height	h_0 cm	2.00				
			Dry weight	W_s g	87.90				
			Height of substance	h_s cm	30.97				
load step	Pressure p kg/cm^2	Consolidation volume Δv ($10^{-3} cm^3$)	Height of specimen h cm	Average height of specimen \bar{h} cm	Compression strain Δe %	Coefficient of volume compression α_v ($10^{-2} cm^2$)	Volume ratio f	Void ratio e	Calculation formula
0	0.00		2.00				0.9009	0.4500	$e_0 = \frac{W_s}{G_s \cdot \gamma_w \cdot V} - 1$
1	0.32	10.20	1.988	1.9244	0.191	16.222	0.8617	0.8617	$\Delta e = \frac{\Delta v}{V_0}$
2	0.64	42.30	1.8565	1.8770	2.253	7.040	0.8202	0.8202	$w = \frac{\Delta v (S_r)}{V} \cdot \frac{1}{100}$
3	1.28	57.90	1.7986	1.8276	3.168	4.95	0.7635	0.7635	$f = \frac{e_0 - \Delta e}{1 + \Delta e}$
4	2.57	61.70	1.7375	1.7651	3.458	2.678	0.7036	0.7036	$e = f - 1$
5	5.135	70.70	1.6674	1.7025	4.117	1.605	0.6348	0.6348	$S_{r0} = \frac{G_s \cdot w}{e_0}$
6	0.64	-21.30	1.6886	1.6780	-1.263	-0.281	0.6556	0.6556	$\bar{p} = \sqrt{p_0 \cdot p_{100}}$
7	0.32	-10.50	1.6997	1.6839	-0.619	-1.054	0.6659	0.6659	\sqrt{t} Method
8	0.00	-51.00	1.7570	1.7351	-3.008	-3.400	0.7168	0.7168	$c_c = \frac{0.258 (\frac{h}{z})^2}{i_{100}}$
9									Curve rule method
10									$c_c = \frac{0.197 (\frac{h}{z})^2}{i_{100}}$
11									$c_c = \frac{\Delta e}{\Delta p} \cdot c_1$
12									$L = \frac{C_c \cdot w \cdot 70}{1000}$
load step	Pressure p kg/cm^2	$0.848 (\frac{h}{z})^2$ $0.197 (\frac{h}{z})^2$	t_{100} min	c_c cm/min	Initial consolidation volume Δv ($10^{-3} cm^3$)	Unit of consolidation ratio $\frac{\Delta v}{\Delta p}$	Coefficient of consolidation c_v cm^2/min	Coefficient of permeability k cm/min	Remarks:
0	0.00								W-5
1	0.32		56.44	0.0093	78.72	0.7776	0.00723	0.1170	
2	0.64		5.76	0.1298	12.78	0.3021	0.0392	0.2759	
3	1.28		10.89	0.095	35.60	0.4421	0.0799	0.0985	
4	2.57		16	0.047	35.11	0.5409	0.02423	0.0644	
5	5.135		9.923	0.062	61.1	0.8776	0.05404	0.0867	
6									
7									Coefficient of compression C_c
8									Yield stress of consolidation p_0 kg/cm^2
9									
							M.d.		Job No.
							Dr.		
							Ch.		

CALCULATION DATA

CONSOLIDATION TEST

PROJECT SILIWANGI PLAZA DATE OF TESTING _____
 LOCATION SEMARANG TESTED BY _____
 BORING NO. 1. -5.00



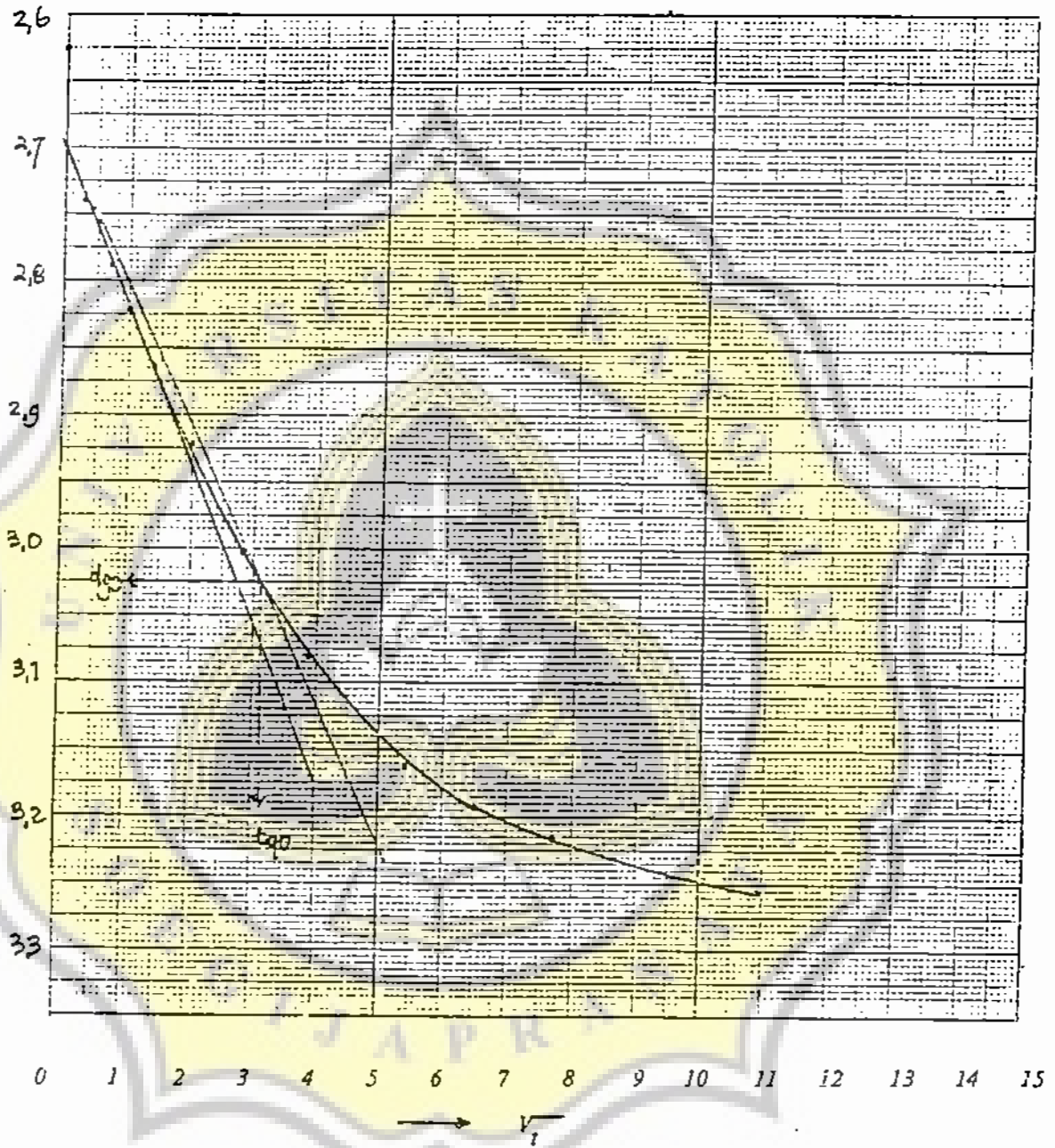
Sample No.	Depth	Liquid limit w _L %	plastic index PI	initial void ratio	coefficient of compression	void stress of consolidation p _v kg/cm ²	Sign
P1.	5.00	-	1.9609	1.3684	0.9482		
					0.5242		
					0.6308		

		M.A.		Job No.
		P.C.		
e - or f - log p REPORTS		Ch.		

CONSOLIDATION TEST

(V_t Curve)

Project SILIWANGI PLAZA
 Location BEKARANG
 Sample No. B.1 Depth -5.00 Date of testing _____
 Soil Description _____ Tested by _____



Load Step	16 kg		
Consolidation pressure kg/cm ²	5,135		
Initial value (d _i)	2,6250		
Compensated initial value (d ₀)	2,700		
d ₉₀	3,250		
Final value d _f	3,3260		
190	9,923		
$\Delta d' = 10/9 (d_0 - d_{90})$	0,611		

CONSOLIDATION TEST

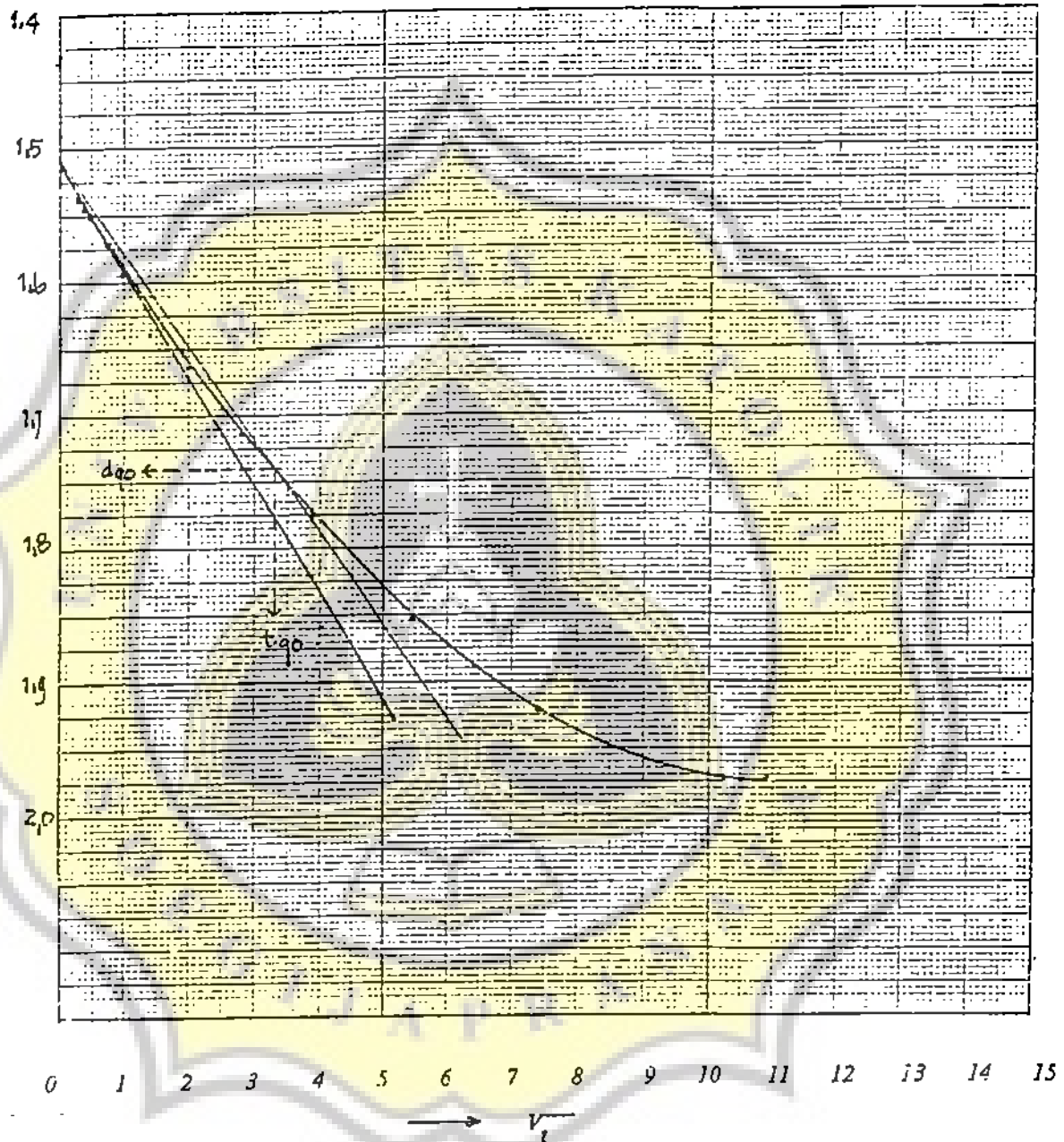
(V_t Curve)

Project Siliwangi Plaza

Location Semarang

Sample No. B.I Depth -5.00 Date of testing _____

Soil Description _____ Tested by _____

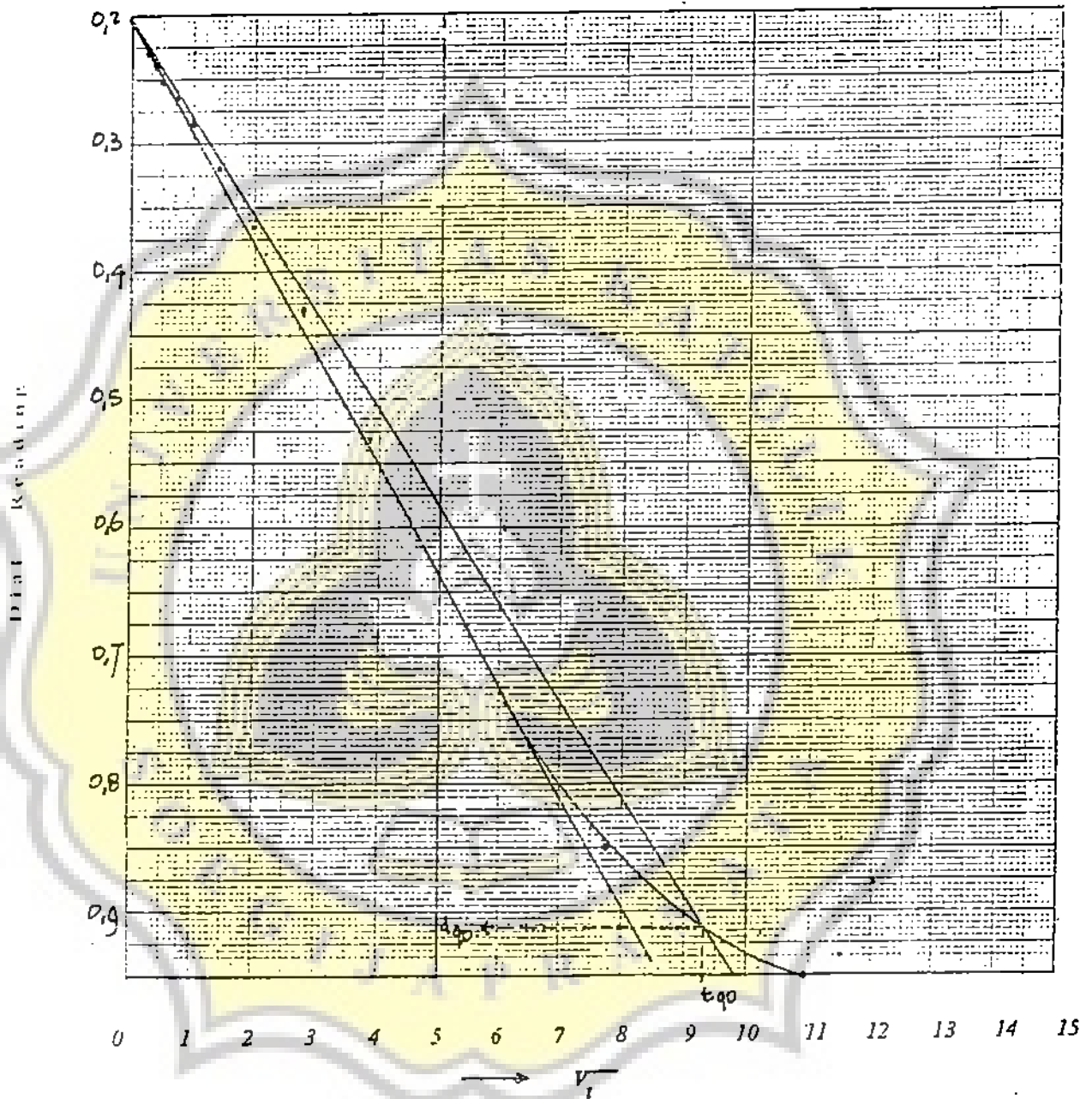


Load Step	4 kg		
Consolidation pressure kg/cm ²	1,28		
Initial value (di)	1,4350		
Compensated initial value (do)	1,510		
d ₉₀	1,740		
Final value df	2,0140		
t ₉₀	10,89		
$\Delta e' = 10/9 (d_o - d_{90})$	0,255		

CONSOLIDATION TEST

(V_1 Curve)

Project SILIWANGI PLAZA
 Location SEMARANG
 Sample No. B.I Depth -5.00 Date of testing _____
 Soil Description _____ Tested by _____

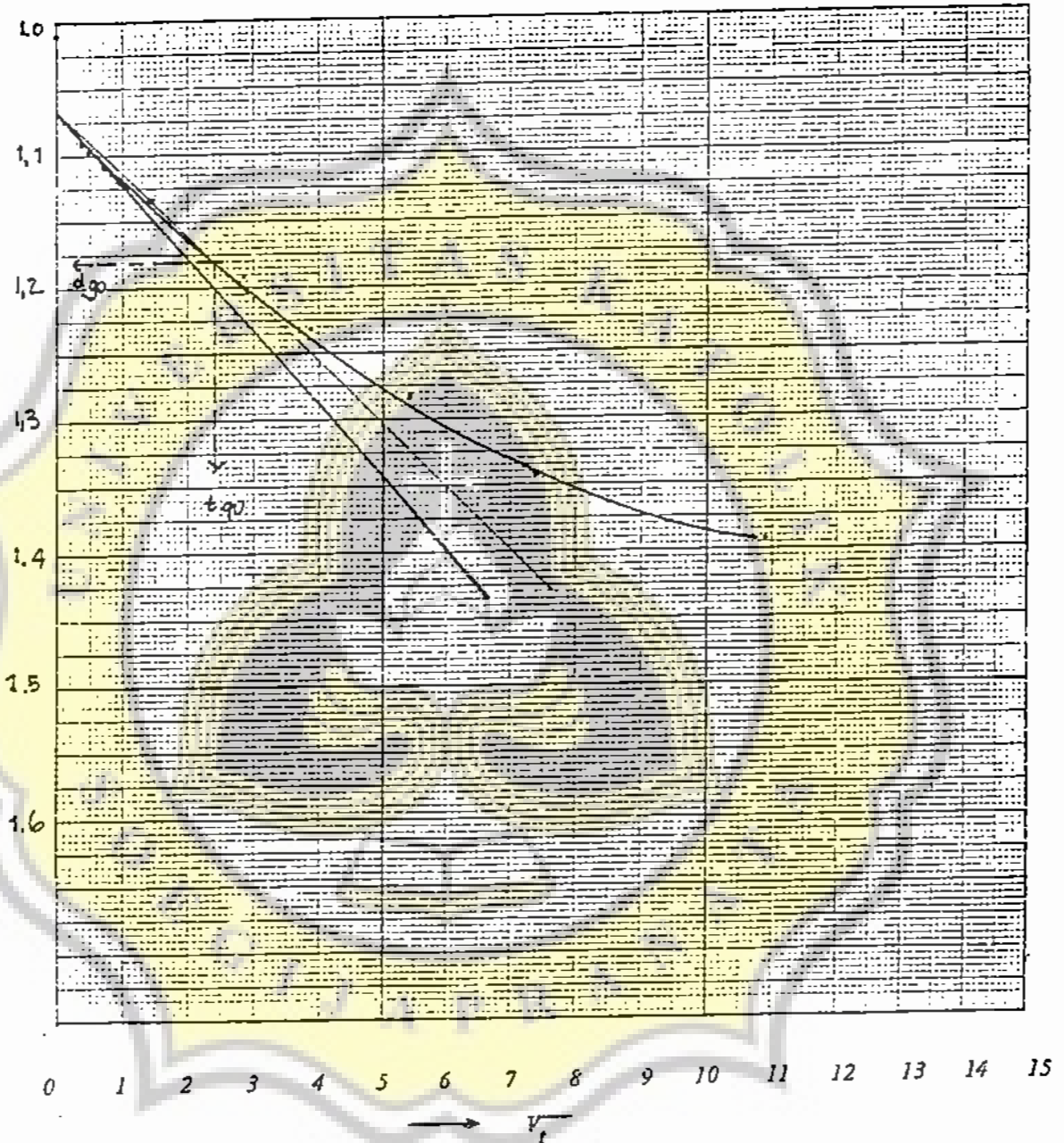


Load Step	1 kg		
Consolidation pressure kg/cm ²	0,32		
Initial value (d _i)	0,000		
Compensated initial value (d ₀)	0,204		
d ₉₀	0,9125		
Final value d _f	1,0120		
t ₉₀	25,49		
$\Delta e' = 10/9 (d_{90} - d_{90})$	0,7272		

CONSOLIDATION TEST

(V_t Curve)

Project SILIWANGI PLAZA
 Location SEMARANG
 Sample No. 3.I Depth -5.00 Date of testing _____
 Soil Description _____ Tested by _____

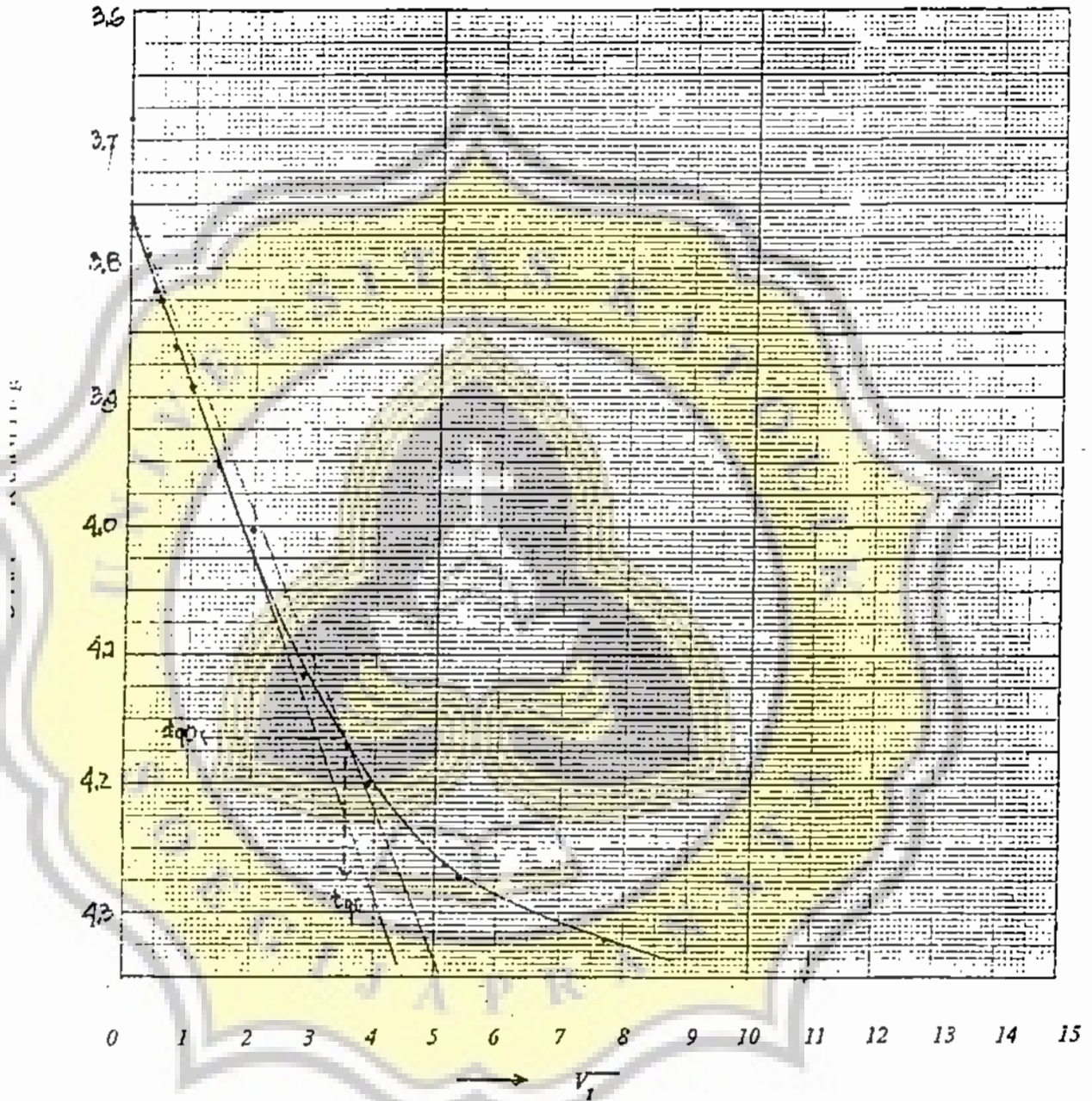


Load Step	2 kg		
Consolidation pressure kg/cm ²	0,64		
Initial value (di)	1,012		
Compensated initial value (do)	1,0650		
d 90	1,180		
Final value df	1,4350		
190	5,76		
$\Delta d' = 10,9 (80-690)$	0,1273		

CONSOLIDATION TEST

(V_t Curve)

Project SILIWANGI PLAZA
 Location SEMARANG
 Sample No. B. 1 Depth 7.00 Date of testing _____
 Soil Description _____ Tested by _____



Load Step	16kg		
Consolidation pressure kg/cm ²	5,135		
Initial value (d_i)	3,6540		
Compensated initial value (d_o)	3,760		
d_{90}	4,1650		
Final value d_f	4,4340		
190	12,603		
$\Delta d' = 10/9 (d_o - d_{90})$	0,45		

CONSOLIDATION TEST

PROJECT SILIWANGI PLAZA DATE OF TESTING _____
 LOCATION SEMARANG TESTED BY _____
 SAMPLE NO. 3.1 Depth -7.00

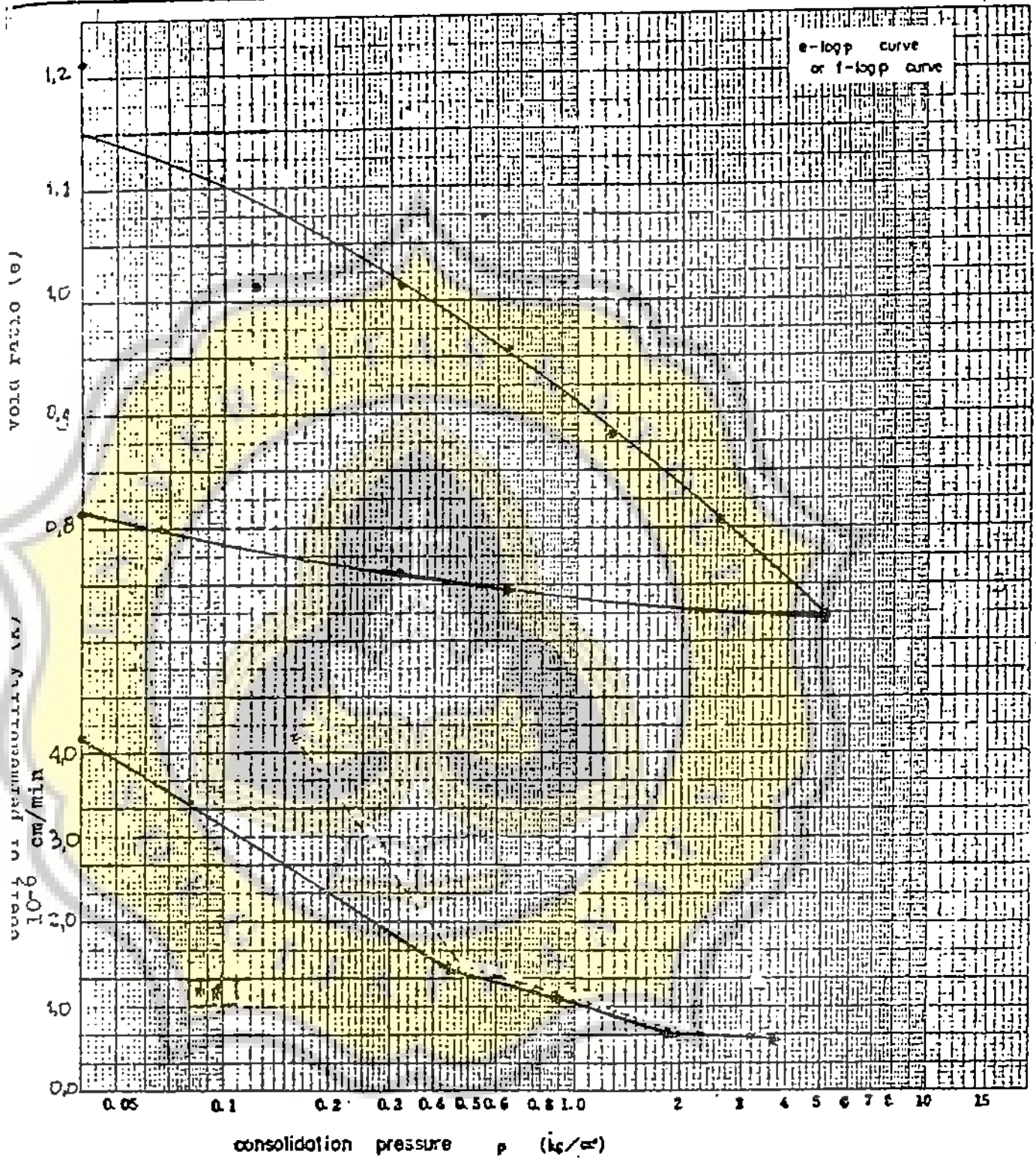
Sample condition		Room temperature (°C)			Tester No.	Ha				
Nature of soil	Soil classification			Section area A cm ²	31.157	Initial water content w ₀ %	40.69			
	Soil gravity G _s	2.7752		Initial height h ₀ cm	2.00	Initial volume ratio f ₀	2.2155			
	Liquid limit w _L %	-		Dry weight W _d g	78.05	Initial void ratio e ₀	1.1290			
	Plastic limit w _p %	-		Height of substance h _s cm	0.9027	Initial degree of saturation S _{w0} %	-			
Load step	Pressure P kN/m ²	ΔP kN/m ²	Consolidation volume ΔV (10 ⁻³ cm ³)	Height of specimen h cm	Average height of specimen \bar{h} cm	Compression strain Δε %	Coefficient of volume compression α _v , α _s	Volume ratio f	Void ratio e	Calculation formula
0	0.00			2.00				2.2155	1.1295	$A_0 = \frac{W_d}{G_s \cdot v_s \cdot A}$
1	0.32	0.32	785.70	1.8149	1.90745	9.704	30.325	2.0705	1.0715	$\Delta e = \frac{\Delta d}{A}$
2	0.64	0.32	48.80	1.7561	1.7005	2.7255	8.5172	1.9555	0.9565	$w = \frac{w_s (\%) \cdot 1}{100}$
3	1.28	0.64	65.70	1.7009	1.7335	3.7400	5.9218	1.8842	0.8842	$f = \frac{A}{A_0}$
4	2.57	1.20	89.30	1.6316	1.6663	4.1500	3.2240	1.8075	0.8075	$e = f - 1$
5	5.135	2.565	74.50	1.5571	1.5944	4.6728	1.8433	1.7249	0.7249	$S_{w0} = \frac{G_s \cdot w_0}{e_0}$
6	0.64	0.32	-20.60	1.5777	1.5674	-1.3123	0.2424	1.7478	0.7478	$E = \sqrt{P_0 \cdot P_{100}}$
7	0.32	0.32	-9.80	1.5875	1.5626	-0.6192	1.935	1.7586	0.7586	\sqrt{t} method
8	0.00	0.32	-50.20	1.6378	1.6126	-3.1891	9.747	1.6143	0.6143	$C_c = \frac{0.848 (\frac{A}{2})^2}{t_{90}}$
9										Curve rule method
10										$C_c = \frac{0.297 (\frac{A}{2})^2}{t_{90}}$
11										$C_c = \frac{\Delta e'}{\Delta d} \cdot C_c$
12										$\lambda = \frac{C_c \cdot (1 - e_0)}{L_{90}}$
Load step	Pressure P kN/m ²	\bar{P} kN/m ²	$0.848 (\frac{A}{2})^2$ $0.197 (\frac{A}{2})^2$	t_{90} min	C_c cm/min	Initial consolidation volume $\frac{\Delta d}{\Delta d}$ (10 ⁻³ cm ³)	Initial consolidation ratio $\frac{\Delta d'}{\Delta d}$	Coefficient of consolidation C_u cm ² /min	Coefficient of permeability k cm/min	Remarks: x 10 ⁻⁶
1	0.00	0.00	0.7713	15.23	0.1396	18.32	0.0063	0.01382	1.1690	
2	0.32	0.163	0.75917	29.16	0.02503	31.70	0.53959	0.016905	1.4402	
3	0.64	0.305	0.63706	21.16	0.0307	42.80	0.65145	0.019609	1.1612	
4	1.28	1.814	0.5859	18.48	0.0317	44.40	0.64063	0.0200310	0.6528	
5	2.57	3.623	0.5389	12.00	0.0427	45.	0.60403	0.025780	0.4754	
6	5.135									
CALCULATION DATA										
										Md.
										Dr.
										Ch.
										Job No.

Coefficient of compression
C_c

Yield stress of consolidation
P_y k₂/cm²

CONSOLIDATION TEST

PROJECT SILIWANGI PLAZA DATE OF TESTING _____
 LOCATION SEMARANG TESTED BY _____
 BORING NO. T-7.00



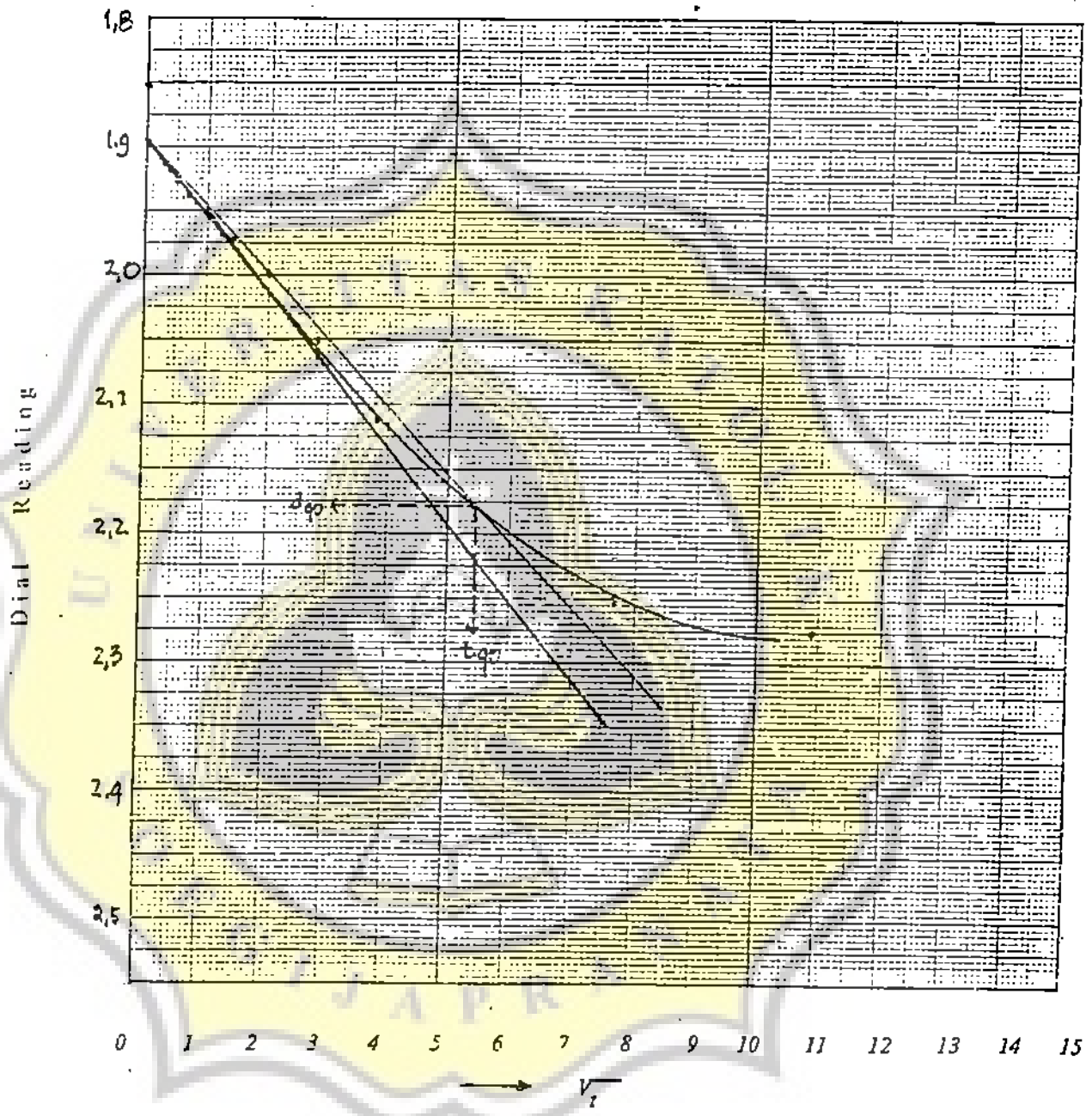
Sample No.	Depth	Liquid limit w _L %	Initial volume ratio f _v	Initial void ratio	Coefficient of compression C _c	Yield stress of consolidation p _r kg/cm ²	Sign
No B 1	-7.00 m.	-	2.2155	1.7290	1.2792		
No					0.6424		
No					0.7576		
No							

	M.C.		Job No.
e- or f- log p REPORTS	Dr.		
	Ch.		

CONSOLIDATION TEST

(V_t Curve)

Project SILIW/PGI PLAZA
 Location SEMARANG.
 Sample No. B. I Depth 2.00 Date of testing _____
 Soil Description _____ Tested by _____

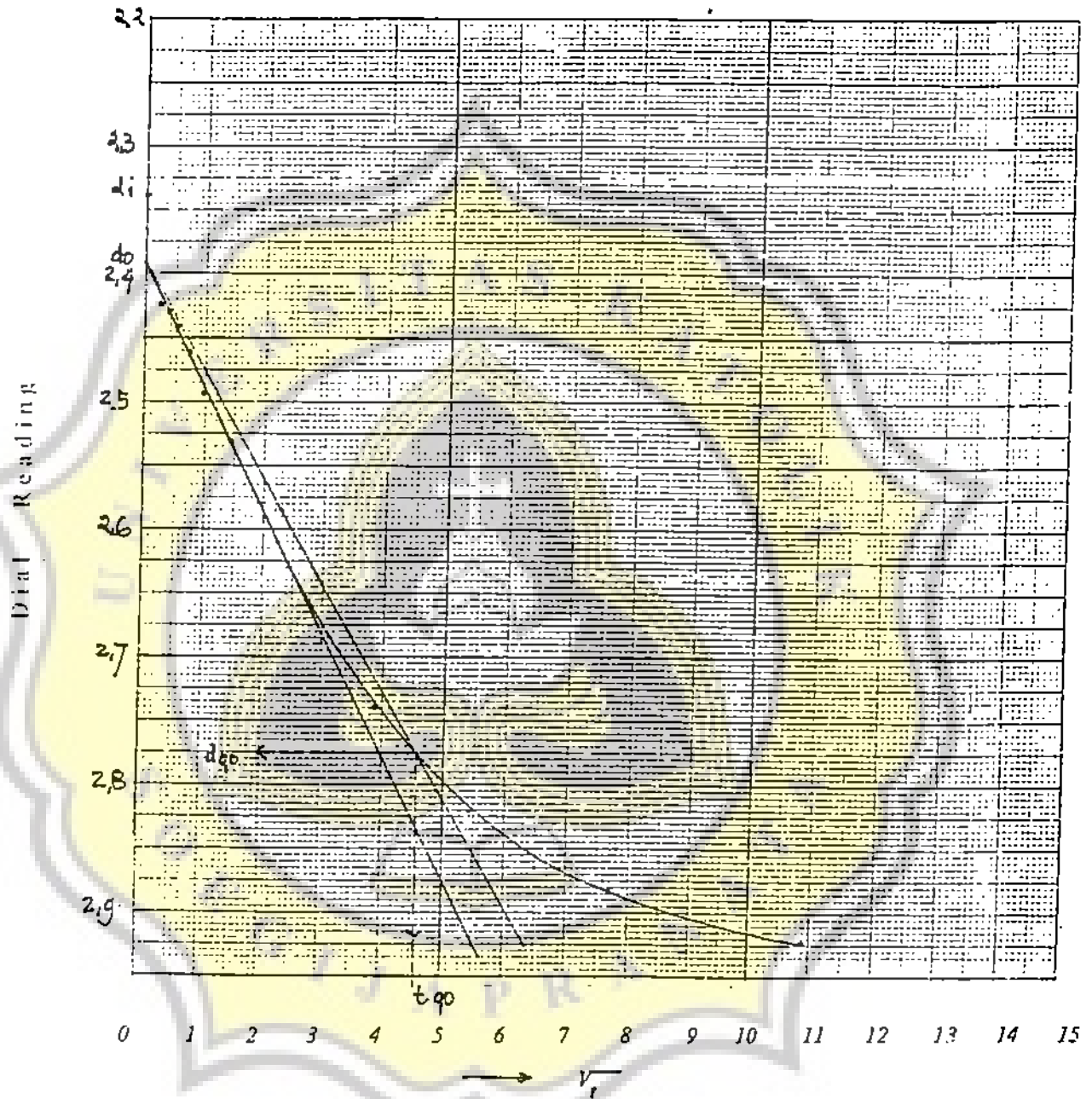


Load Step	2 kg		
Consolidation pressure kg/cm ²	0,64		
Initial value (d _i)	1,8510		
Compensated initial value (d ₀)	1,8950		
d ₉₀	2,180		
Final value d _f	2,3390		
t ₉₀	29,16		
$\Delta d' = 10/9 (d_0 - d_{90})$	0,317		

CONSOLIDATION TEST

(V_r Curve)

Project SILIWANGI PLAZA
 Location SEMARANG.
 Sample No. B. I Depth 7.00 Date of testing _____
 Soil Description _____ Tested by _____



Load Step	4 kg		
Consolidation pressure kg/cm ²	1,28		
Initial value (d _i)	2,3390		
Compensated initial value (d ₀)	2,390		
d ₉₀	2,775		
Final value d _f	2,9960		
t ₉₀	21,16		
$\Delta d' = 10/9 (d_0 - d_{90})$	0,428		

PERPUSTAKAAN

CONSOLIDATION TEST

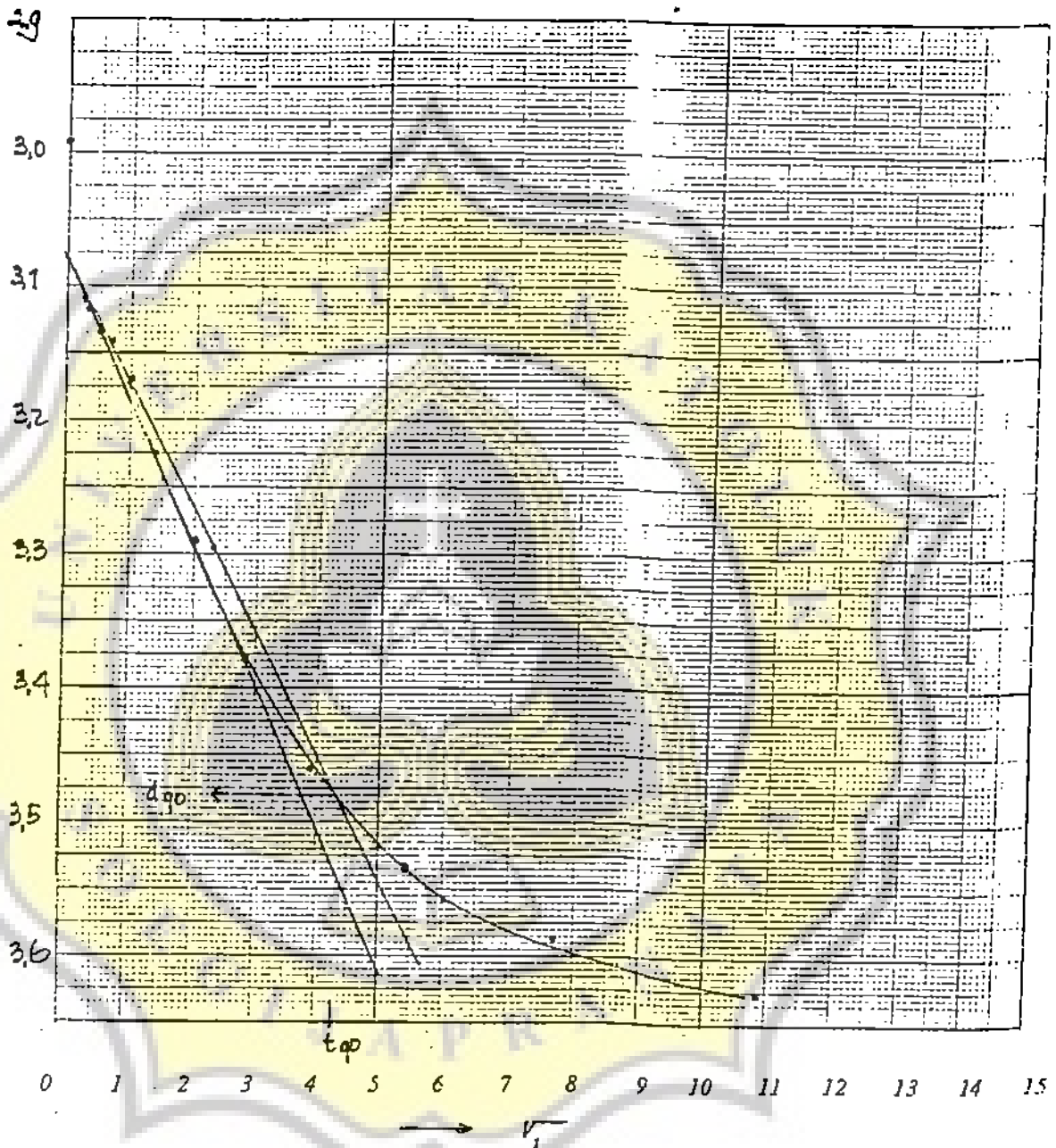
(V_1 Curve)

Project SILIWANGI PLAZA

Location SEMARANG

Sample No. B. I Depth 7.00 Date of testing _____

Soil Description _____ Tested by _____

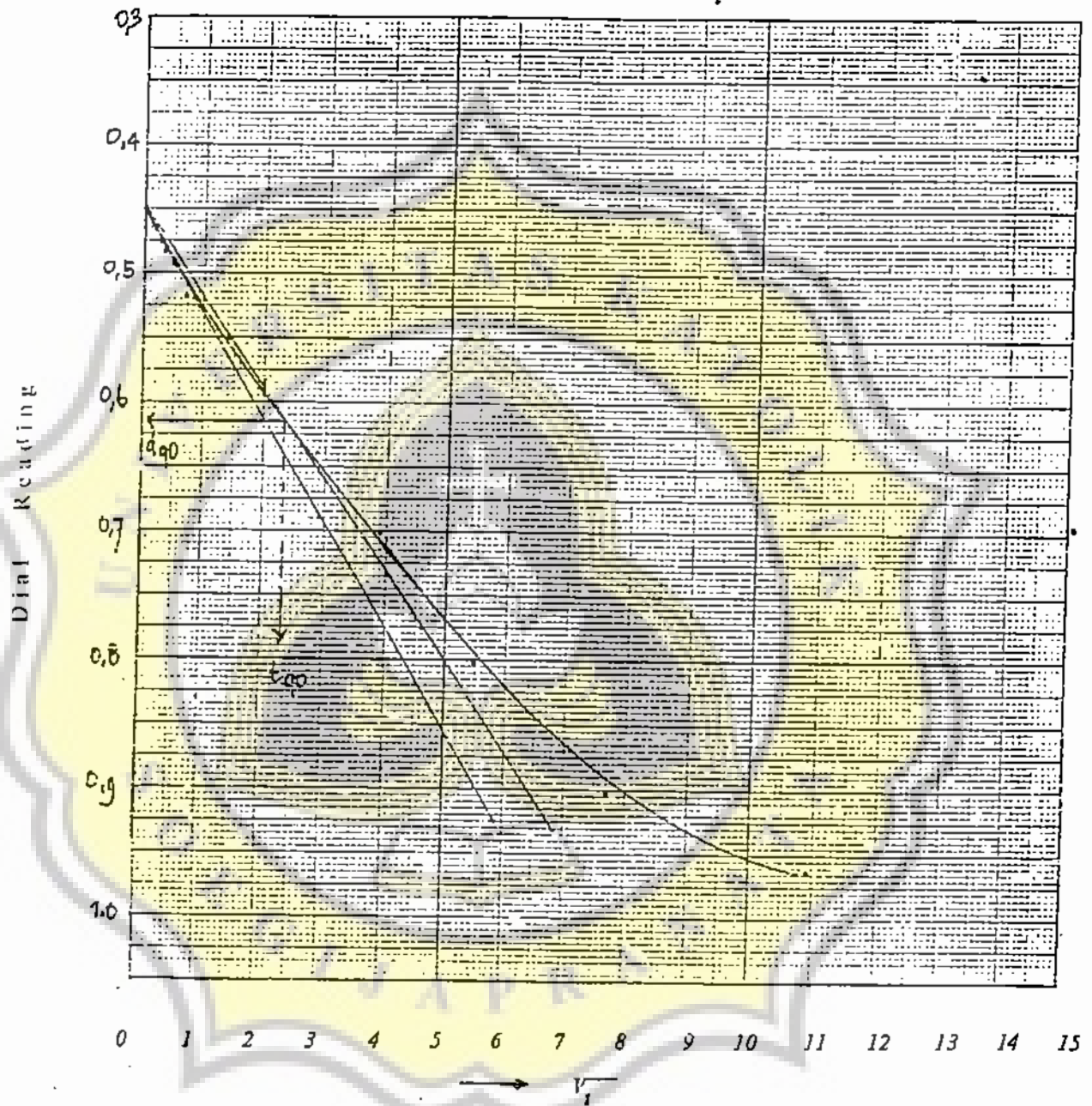


Load Step	8 kg		
Consolidation pressure kg/cm ²	2,57		
Initial value (di)	2,9960		
Compensated initial value (do)	3,080		
d 90	3,480		
Final value df	3,6890		
i90	18,49		
$\Delta d^* = 10/9 (d_o - d_{90})$	0,444		

CONSOLIDATION TEST

(V_1 Curve)

Project SILIWANGI PLAZA
 Location SEMARANG.
 Sample No. B. I Depth 7.00 Date of testing _____
 Soil Description _____ Tested by _____



Load Step	1kg		
Consolidation pressure kg/cm ²	0,32		
Initial value (di)	0,000		
Compensated initial value (do)	0,450		
d 90	0,6150		
Final value df	1,6378		
t90	5,523		
$\Delta d = 10/9 (d_0 - d_{90})$	0,1833		



FORMULIR HIMPUNAN PERHITUNGAN LALU LINTAS
SELAMA 24 JAM (FORMULIR LAPANGAN)

NO. PROPINSI

NAMA PROPINSI JAWA TENGAH

KLAS / NO. POS

LOKASI POS

TANGGAL

HARI KERJA

ARAH LALU LINTAS
DARI:

KE:

GOLONGAN	1	2	3	4	5	6	7	8
JAM	SEPEDA MOTOR SEKUTER SEPEDA KUMBANG DAN RODA 3	SEDAN, JEEP DAN STATION WAGON	OPLET, PICKUP, OPLET, SUBURBAN COMBI & MINIBUS	MICROTRUK	BUS	TRUK 2 SUMBU	TRUK 3 SUMBU ATAU LEBIH, GANDENGAN, TRAILER	KENDARAAN TIDAK BERMOTOR & SEPEDA
06 - 07	481		552	49	86	136	58	274
07 - 08	883		971	98	148	272	103	394
08 - 09	962		1104	87	171	256	115	547
09 - 10	770		994	78	137	217	92	288
10 - 11	577		662	59	103	163	69	279
11 - 12	577		662	59	103	163	69	279
12 - 13	577		662	59	103	163	69	279
13 - 14	577		662	59	103	163	69	279
14 - 15	770		994	78	137	217	92	288
15 - 16	754		1130	81	157	242	112	526
16 - 17	860		1145	67	161	269	98	404
17 - 18	688		916	65	123	215	90	323
18 - 19	516		687	41	97	161	67	226
19 - 20	344		637	32	65	108	45	154
20 - 21	258		458	24	48	81	37	71
21 - 22	172		286	16	32	54	22	61
22 - 23	86		114	8	16	24	11	41
23 - 24	48		57	4	8	12	6	21
24 - 01	20		55	5	9	14	6	13
01 - 02	20		55	5	9	14	6	13
02 - 03	20		55	5	9	14	6	13
03 - 04	20		55	5	9	14	6	13
04 - 05	77		88	8	14	22	9	14
05 - 06	241		276	25	43	68	29	137
TOTAL	10.298		13.327	1.025	1.891	3.062	1.298	4.987

CATATAN :

Data tersebut di atas kami survey
di jalan Siliwangi, arah kendaraan
dari dan ke Semarang.

PENGAWAS :

(.....)

